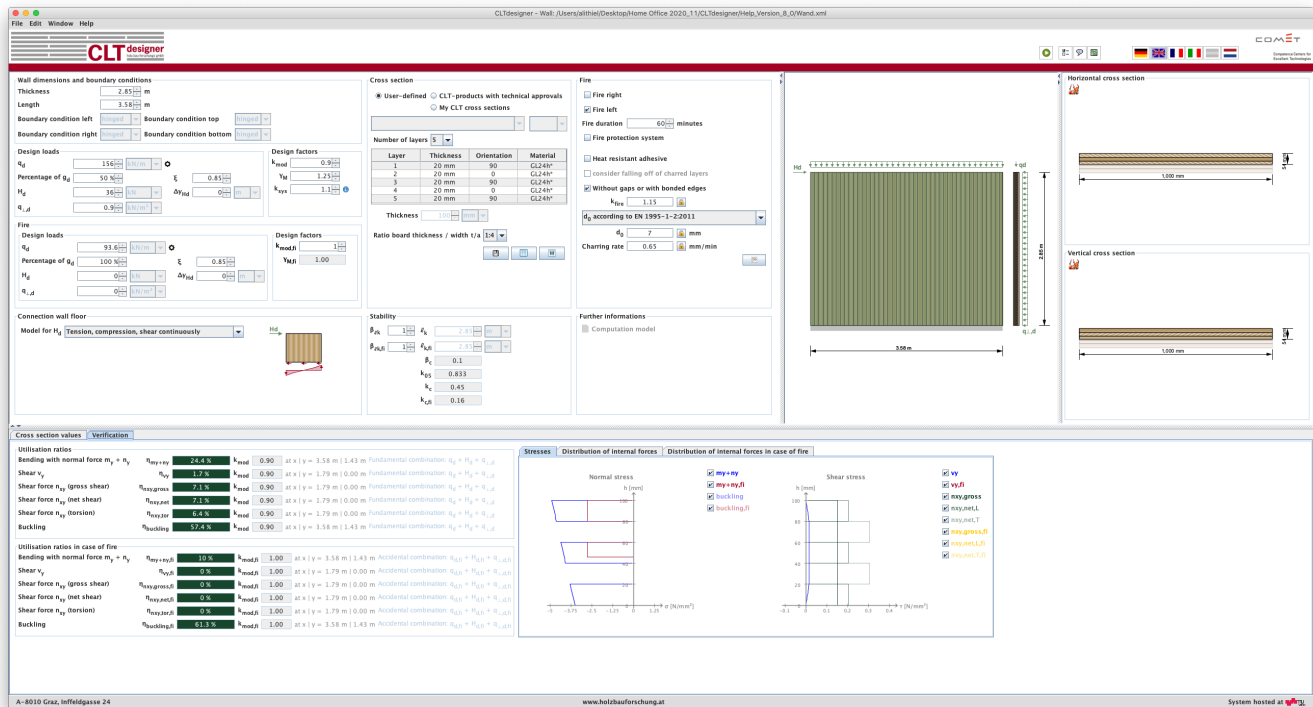


Module "Wall"



Input data

The input is divided into:

- wall dimensions and boundary conditions
- cross section
- design loads and design factors
- stability
- connection wall floor
- fire

Wall dimensions and boundary conditions

In the current version, only rectangular wall elements without openings can be calculated. The wall is defined via a wall height and wall length.

Wall dimensions and boundary conditions

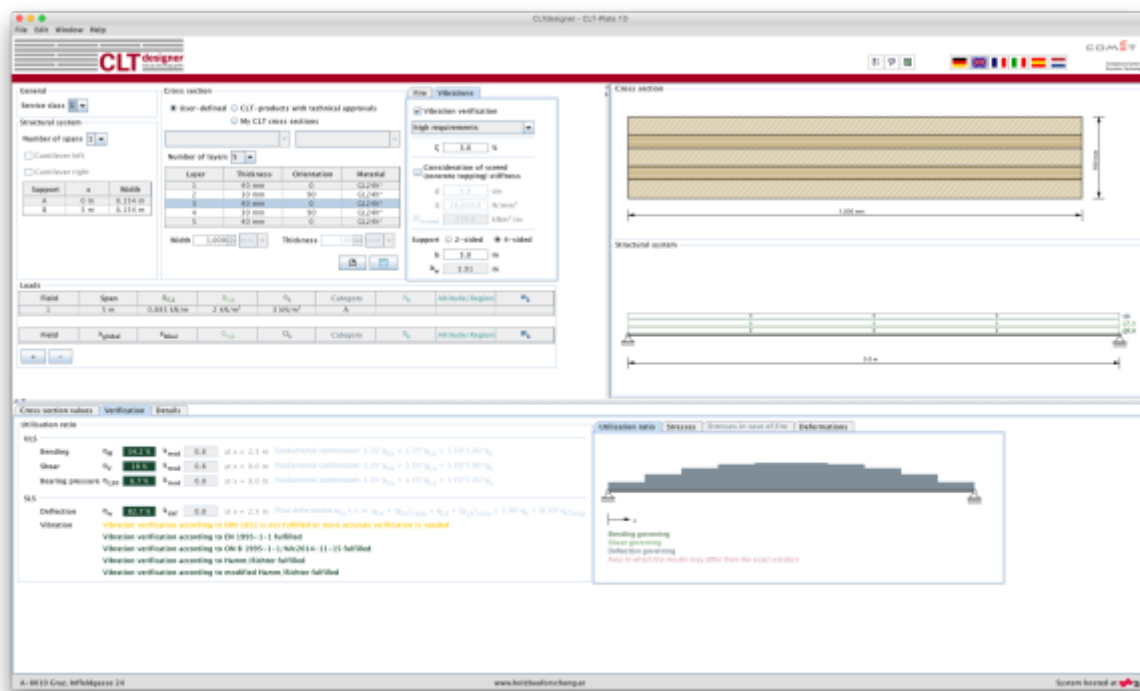
Thickness	2.85	m
Length	3.58	m
Boundary condition left	hinged	
Boundary condition right	hinged	
Boundary condition top	hinged	
Boundary condition bottom	hinged	

The bearing of the wall at the top and bottom edges is hinged and the lateral edges are free. These boundary conditions cannot be changed at present.

Cross section

In this module, the cross section is defined in the direction of the wall length (vertical cross section) and the width of the cross section cannot be changed.

See Module "CLT-Plate 1D - Continuous beam"

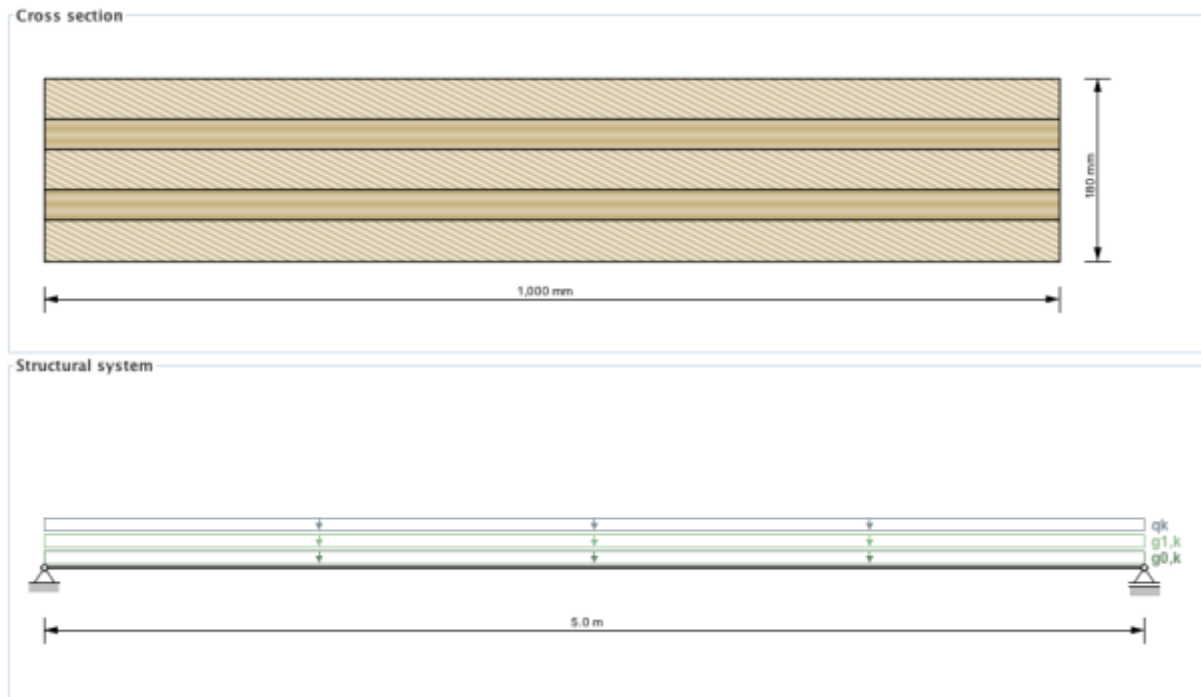


Input data

The input is divided into:

- general information about the project and the considered structural element
- definitions of the structural system
- definitions of the cross section
- input of the loads
- informations about fire and vibration parameters

A graphical representation of the input data is shown on the right side. This offers the possibility for a fast check of the input data.



General

The input field "General" defines the service class. It is only allowed to use CLT elements in areas of service class 1 and 2.

- Service class 1 (interior service condition) is in general consistent with a common utilisation of living spaces.
- Service class 2 (protected exterior service condition) is generally used for open but roofed structures.

General

Service class

Structural system

In the current version a continuous beam with a maximum of 7 spans including a cantilever on the left and right side can be analysed. The supporting width and span of field (via x-value in the table) can be defined within this input field.

Structural system

Number of spans 1

☐ Cantilever left

☐ Cantilever right

Support	x	Width
A	0 m	0.154 m
B	5 m	0.154 m

Cross section

The cross section can be defined by the user or by choosing a typical cross section of a proprietary CLT product. There is also the possibility to save own CLT cross sections in a library. The elements are subdivided by the number of layers.

If a user-defined cross section is entered, the thickness and orientation of each layer can be changed. Furthermore, the material can be changed for all layers. The thickness of each layer has to be within the range of 6.0 mm to 45 mm. In the case of proprietary CLT products, the strength class of lumber and the orientation can be changed. If the orientation is changed, the whole cross section is rotated.

Cross section

☒ User-defined
☐ CLT-products with technical approvals
☐ My CLT cross sections

Number of layers 5

Layer	Thickness	Orientation	Material
1	40 mm	0	GL24h*
2	20 mm	90	GL24h*
3	40 mm	0	GL24h*
4	20 mm	90	GL24h*
5	40 mm	0	GL24h*

Width 1,000 mm
Thickness 160 mm


Ratio board thickness / width t/a 1:4


Beta! Optimise cross section...

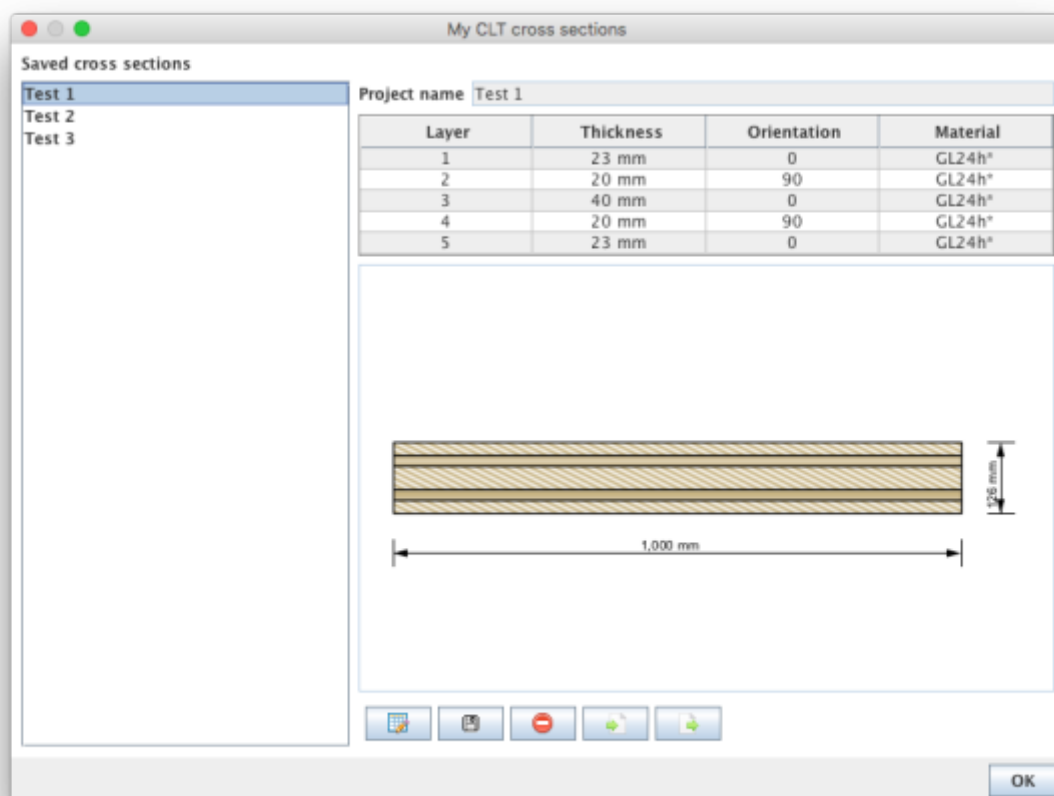
The width of the CLT plate strips can be also defined in this field. The default value is set to 1 m. The thickness of the CLT plate is calculated automatically based on the thickness of the single layers.






The ratio of board thickness to board width can also be changed here. The default setting is 1:4.

My CLT cross sections

By clicking the button  the current cross section can be stored in the library and be retrieved by selecting "My CLT cross sections" later on.

The library can be displayed with the button .



- The edit mode can be entered by clicking on . Currently, only the name of the stored cross section can be changed.
- With  the changes are saved.
- With  the chosen cross section in the sidebar can be removed from the library.
- With  cross sections from a csv file can be imported.
- With  the cross sections from the library can be exported to a csv file.



Syntax of the csv file

name;number of layers n ;layer thickness in [m] t_1 to t_n ;orientation of the layers o_1 to o_n (0


or 90);name of material

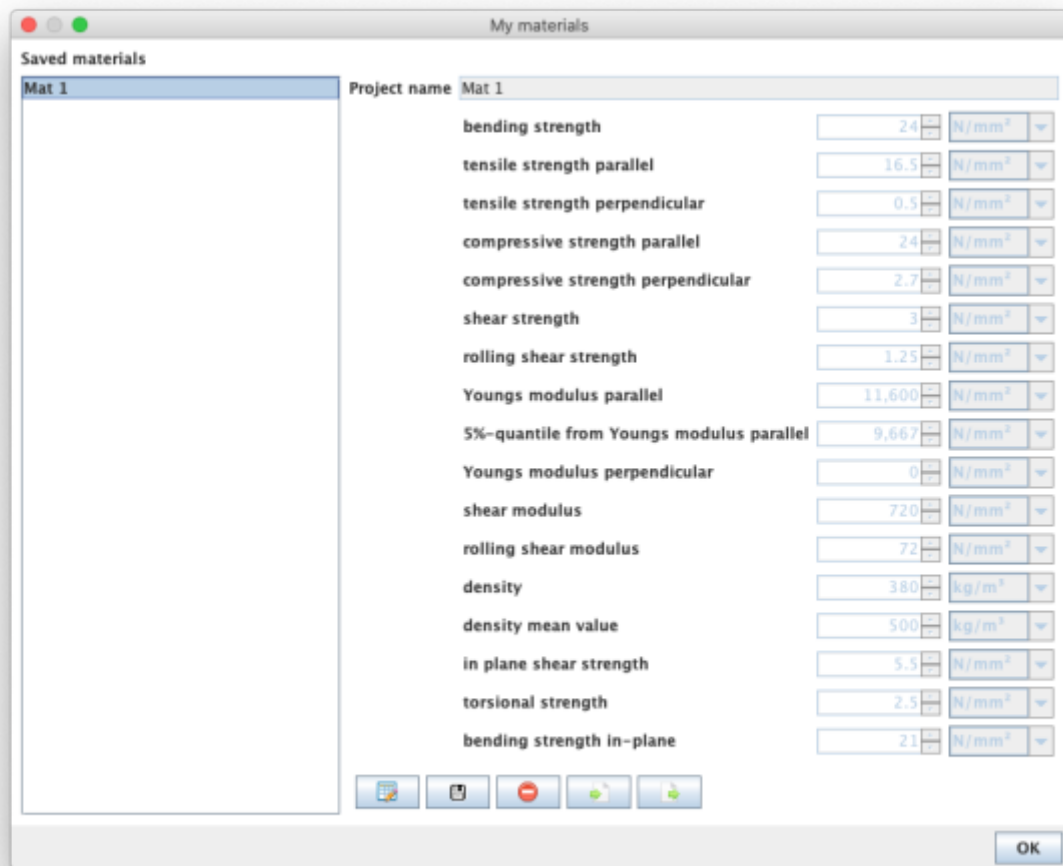


Example:






Test layup;5;0.03;0.02;0.02;0.02;0.03;90;0;90;0;90;GL24h*

My materials

With the button  the material library can be displayed.



Property	Value	Unit
bending strength	24	N/mm ²
tensile strength parallel	16.5	N/mm ²
tensile strength perpendicular	0.5	N/mm ²
compressive strength parallel	24	N/mm ²
compressive strength perpendicular	2.7	N/mm ²
shear strength	3	N/mm ²
rolling shear strength	1.25	N/mm ²
Youngs modulus parallel	11,600	N/mm ²
5%-quantile from Youngs modulus parallel	9,667	N/mm ²
Youngs modulus perpendicular	0	N/mm ²
shear modulus	720	N/mm ²
rolling shear modulus	72	N/mm ²
density	380	kg/m ³
density mean value	500	kg/m ³
in plane shear strength	5.5	N/mm ²
torsional strength	2.5	N/mm ²
bending strength in-plane	21	N/mm ²

- With  the edit mode can be entered.
- With  the changes are saved.
- With  the chosen material in the sidebar can be removed from the library.
- With  materials from a csv file can be imported.
- With  the materials from the library can be exported to a csv file.



Syntax of the csv file

1. row: description of the parameters

2. row: units of the parameters

3. row: value

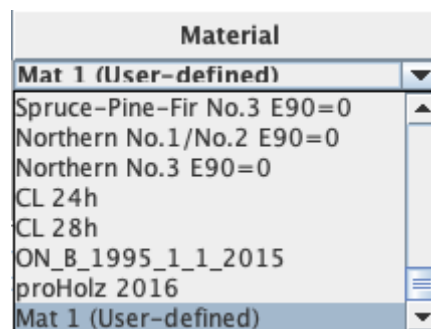
delimiter: ";"



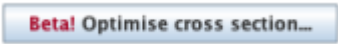
Example:

Name;f_m,k;f_t,0,k;f_t,90,k;f_c,k;f_c,90,k;f_v,k;f_r,k;E_0;E_0,05;E_90;G;G_r;rho_k;rho_mean;f_v,k,IP;f_T,k;f_m,k,IP
;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;kg/m3;kg/m3;N/mm2;N/mm2;N/mm2
Mat 1;24;16.5;0.5;24;2.7;3;1.25;11600;9667;0;720;72;380;500;5.5;2.5;21

The user-defined materials are then displayed in the material selection list.



Optimization of layup

Use the button  to display the window for layup optimization.

Optimisation

Consider in the optimisation:

Producer

☐ best wood SCHNEIDER ☒ KLH

☐ Cross Timber Systems ☐ Mayr-Melnhof Holz

☐ Derix ☐ Piveteaubois

☐ Eugen Decker ☐ Stora Enso

☐ Hasslacher

Number of layers

☐ 3 ☒ 5 ☐ 6 ☒ 7 ☐ 8 ☐ 9 ☐ 11

Plate thickness

min mm max mm

Saved cross sections

☐ My CLT cross sections

Options

☐ Outer cross layers ☒ Double layers

☐ Vibration verification according to EN

Start **Stop**

Producer	Cross section	Plate thickness	Governing proof	Utilisation ratio
KLH	180mm 5s DL	180 mm	Vibration	99.2 %
KLH	190mm 5s DL	190 mm	Vibration	95.4 %
KLH	200mm 5s DL	200 mm	Vibration	90.1 %
KLH	220mm 7s DL	220 mm	Vibration	85.3 %
KLH	240mm 7s DL	240 mm	Vibration	78.4 %
KLH	180mm 7ss DL	180 mm	Vibration	93.9 %
KLH	200mm 7ss DL	200 mm	Vibration	82.8 %
KLH	220mm 7ss DL	220 mm	Vibration	73.1 %
KLH	240mm 7ss DL	240 mm	Vibration	65.8 %
KLH	260mm 7ss DL	260 mm	Vibration	61.6 %
KLH	280mm 7ss DL	280 mm	Vibration	58.2 %

Choose the selected cross section

With the help of this tool, the possible layouts can be determined for the given system and load situation. The optimization can be restricted with regard to producers, number of layers or by means of limits for the panel thickness. Furthermore, outer cross layers or double layers can be included or excluded. With the option "Vibration verification according to EN" the base document is included in the vibration check or not.

With the buttons "Start" and "Stop" the calculation is controlled. Please be patient, depending on the selected parameter the calculation may take a little longer.

The possible setups are then displayed in the table and the selected setup can be transferred to the main window by clicking the "Choose the selected cross section" button.

Loads

The loads are divided into the dead load (weight of the plate) ($g_{0,k}$), permanent loads ($g_{1,k}$), imposed load (q_k), snow load (s_k) and wind load (w_k). This classification is necessary to automatically carry out calculations for different load case combinations.

The plate weight is calculated automatically. The calculation method can be selected in the settings/preferences window. The default calculation method is in accordance with ON B 1991-1-1. A unit weight of 5.5 kN/m^3 is assumed in the calculation. However, the unit weight may also be

calculated using:

- calculation based on the mean value of density of the chosen material
- calculation based on a user-defined density

When entering the imposed loads, one of the following categories has to be chosen:

- A: Areas for domestic and residential activities
- B: Office areas
- C: Areas where people may congregate (with the exception of areas defined under category A, B and D)
- D: Shopping areas
- E: Areas for storage and industrial activities
- F: Traffic and parking areas for light-duty vehicles
- G: Traffic and parking areas for medium-duty vehicles
- H: Roofs

When entering the snow load, the country code or an altitude above sea level where the structure will be located has to be specified:

- < 1000 m
- > 1000 m
- FIN, IS, N, S

The span of each field can also be modified in the table of distributed loads.

Concentrated loads can be entered in the second table. The position can be defined whether by the local or global x-coordinate.

Loads								
Field	Span	$g_{0,k}$	$g_{1,k}$	q_k	Category	s_k	Altitude/Region	w_k
1	3.5 m	0.55 kN/m	0.58 kN/m ²	1.2 kN/m ²	A			
2	4 m	0.55 kN/m	0.58 kN/m ²	1.2 kN/m ²	A			
3	4.25 m	0.55 kN/m	0.58 kN/m ²	1.2 kN/m ²	A			

Field	x_{global}	x_{local}	$G_{1,k}$	Q_k	Category	s_k	Altitude/Region	w_k

Fire

By choosing "Fire above" and/or "Fire below" in the tab "Fire" a structural fire design has to be carried out. The "Fire duration" is specified in minutes and can be increased (or decreased) by increments of 30 minutes by pressing the up (or down) arrows, or defined by entering a specific duration between 0 minutes and 240 minutes in the allotted box. By ticking the box next to "Fire protection system" a layer of fire protection is added to the plate. Furthermore, the position ("above" and/or "below") must be declared and the parameters t_{ch} , t_f , k_2 and k_3 must be specified. In case the failure time of the fire protection system is equal to the time until the protected component starts to burn, the option " $t_f = t_{ch}$ " shall be checked.

Fire

Vibrations

☐ Fire above

☒ Fire below

Fire duration minutes

☒ Fire protection system

☒ below

t_{ch} minutes

t_f minutes ☒ $t_f = t_{ch}$

k_2

k_3

☐ Heat resistant adhesive

☒ consider falling off of charred layers

☐ Without gaps or with bonded edges

k_{fire}

d_0 mm

Charring rate mm/min

For a user-defined cross section, options are given for specifying heat resistant adhesives, presence of grooves, and whether the layers are edge-glued. For CLT products both values are set automatically and they cannot be changed.

Some manufacturers offer CLT elements with different adhesives, so instead of selecting "Heat resistant adhesive", it is also possible to select the adhesive.

Adhesive ☒ PUR ☐ MUF

The values k_{fire} (conversion factor 20%-quantiles) and d_0 (layer thickness to take into consideration the influence of temperature exposure) are pre-set and cannot be changed. The charring rate is dependent on the option edge glued or without groove. For a user-defined cross section this value can be changed.

In some approvals or design proposals a different charring rate is expected from the 2nd layer onwards. This will show up as follows:

Charring rate from 2nd layer onwards mm/min

Vibrations

The tab "Vibrations" allows for vibration verification.

Fire
Vibrations

☒ **Vibration verification**

normal requirements ▼

ζ

3.0

 %

☒ **Consideration of screed (concrete topping) stiffness**

d

6.0

 cm
 E

26,000.0

 N/mm²
 EI_{screed}

468

 kNm²/m

Support ☐ 2-sided ☒ 4-sided

b

5.0

 m

b_w

3.13

 m

For the vibration verification the following specifications are of importance:

- high or normal requirements? This choice will have an influence on the limit values.
- modal damping factor
- consideration of the screed (concrete topping) stiffness
 - thickness of the screed (concrete topping)
 - modulus of elasticity of the screed (concrete topping)
- support (2-sided or 4-sided)
- room width b perpendicular to the load carrying direction

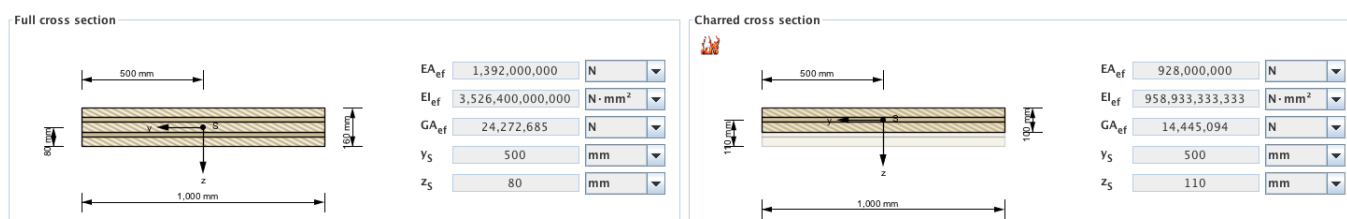
The effective width b_w of the chosen cross section used by the stiffness criteria will be specified.

Results and output

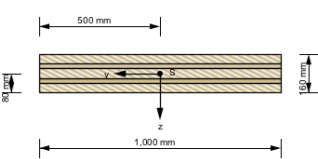
Load combinations are compiled based on the input loads entered in the "Loads" field. The respective k_{mod} - and k_{def} -values can be determined automatically based on the classification of loads (plate weight, wind load, etc.).

Cross section values

Output values generated in the tab "Cross section values" field include the effective stiffness (depending on the chosen calculation method), the position of the centre of mass for the full cross section and also for the charred cross section in case of structural fire design.



Full cross section



EA

1,392,000,000

N

$EI_{yy,A}$

185,600,000,000

N·mm²

$EI_{yy,B}$

3,340,800,000,000

N·mm²

EI_{yy}

3,526,400,000,000

N·mm²

$GA_{z,A}$

∞

N

$GA_{z,B}$

21,600,000

N

GA_z

21,600,000

N

y_s

500

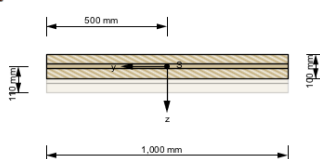
mm

z_s

80

mm

Charred cross section



EA

928,000,000

N

$EI_{yy,A}$

123,733,333,333

N·mm²

$EI_{yy,B}$

835,200,000,000

N·mm²

EI_{yy}

958,933,333,333

N·mm²

$GA_{z,A}$

∞

N

$GA_{z,B}$

10,800,000

N

GA_z

10,800,000

N

y_s

500

mm

z_s

110

mm

Summary of the results

A summary of the verifications can be retrieved via the tab "Verifications". The utilisation ratios for various limit states are colour-coded indicating if the verification is fulfilled (green), not fulfilled (red) or a more accurate verification is needed (yellow). The locations of the maximum utilisation ratio and the governing combinations are compiled in the same way.

Cross section values

Verification

Details

Utilisation ratio

ULS

Bending

η_M

28.3 %

k_{mod}

0.8

at x = 7.5 m

Fundamental combination: $1.35 \cdot g_{0,k} + 1.35 \cdot g_{1,k} + 1.50 \cdot 1.00 \cdot q_k$

Shear

η_V

13 %

k_{mod}

0.8

at x = 7.5 m

Fundamental combination: $1.35 \cdot g_{0,k} + 1.35 \cdot g_{1,k} + 1.50 \cdot 1.00 \cdot q_k$

Bearing pressure

$\eta_{c,90}$

5 %

k_{mod}

0.8

at x = 7.5 m

Fundamental combination: $1.35 \cdot g_{0,k} + 1.35 \cdot g_{1,k} + 1.50 \cdot 1.00 \cdot q_k$

SLS

Deflection

η_w

75.8 %

k_{def}

0.85

at x = 9.83 m

Final deformation $w_{tot,fin} \ t = \infty$: $g_{0,k} + (g_{0,k})^{creep} + g_{1,k} + (g_{1,k})^{creep} + 1.00 \cdot q_k + (0.30 \cdot q_k)^{creep}$

Vibration

Vibration verification according to DIN 1052 fulfilled

Vibration verification according to EN 1995-1-1 fulfilled

Vibration verification according to ON B 1995-1-1/NA:2009-07 fulfilled

Vibration verification according to Hamm/Richter fulfilled

Vibration verification according to modified Hamm/Richter fulfilled

Utilisation ratio in case of fire

ULS

Bending

$\eta_{M,fi}$

15.7 %

k_{mod}

1.0

at x = 7.5 m

Accidental combination: $g_{0,k} + g_{1,k} + 0.30 \cdot q_k$

Shear

$\eta_{V,fi}$

6.1 %

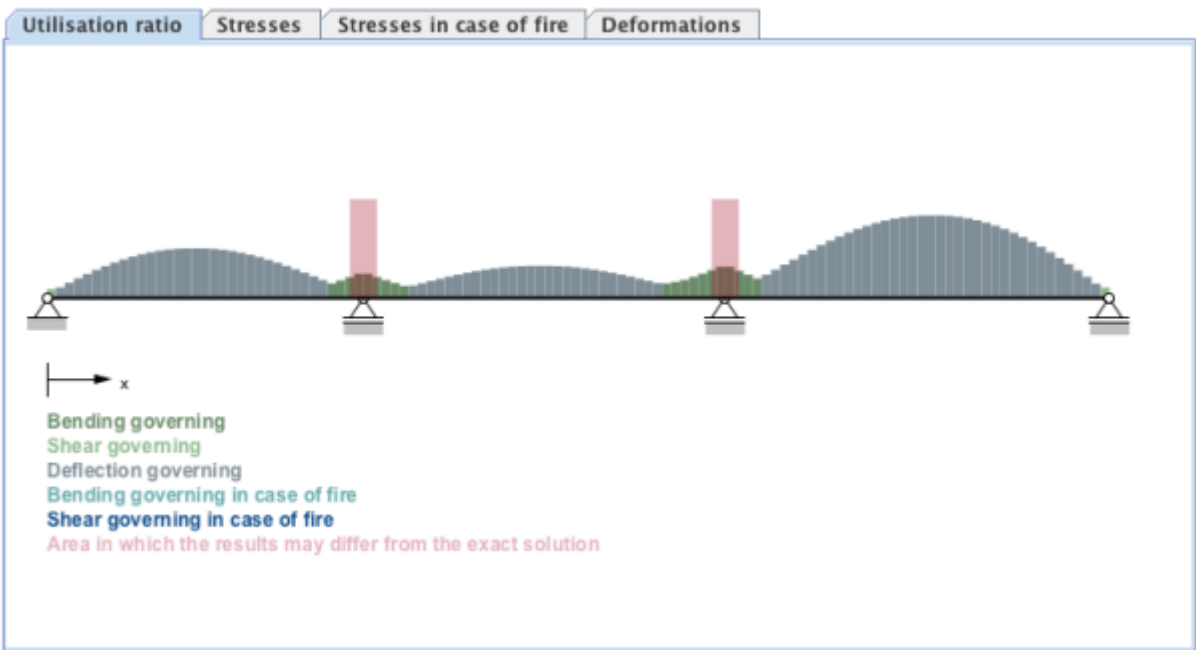
k_{mod}

1.0

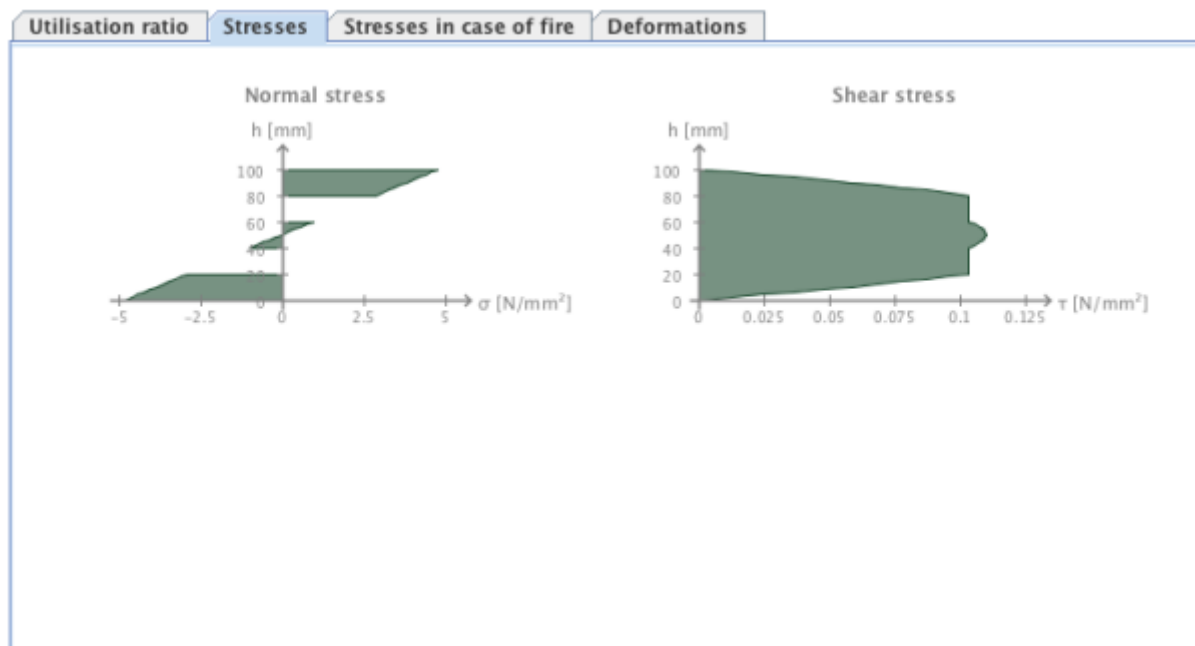
at x = 7.5 m

Accidental combination: $g_{0,k} + g_{1,k} + 0.30 \cdot q_k$

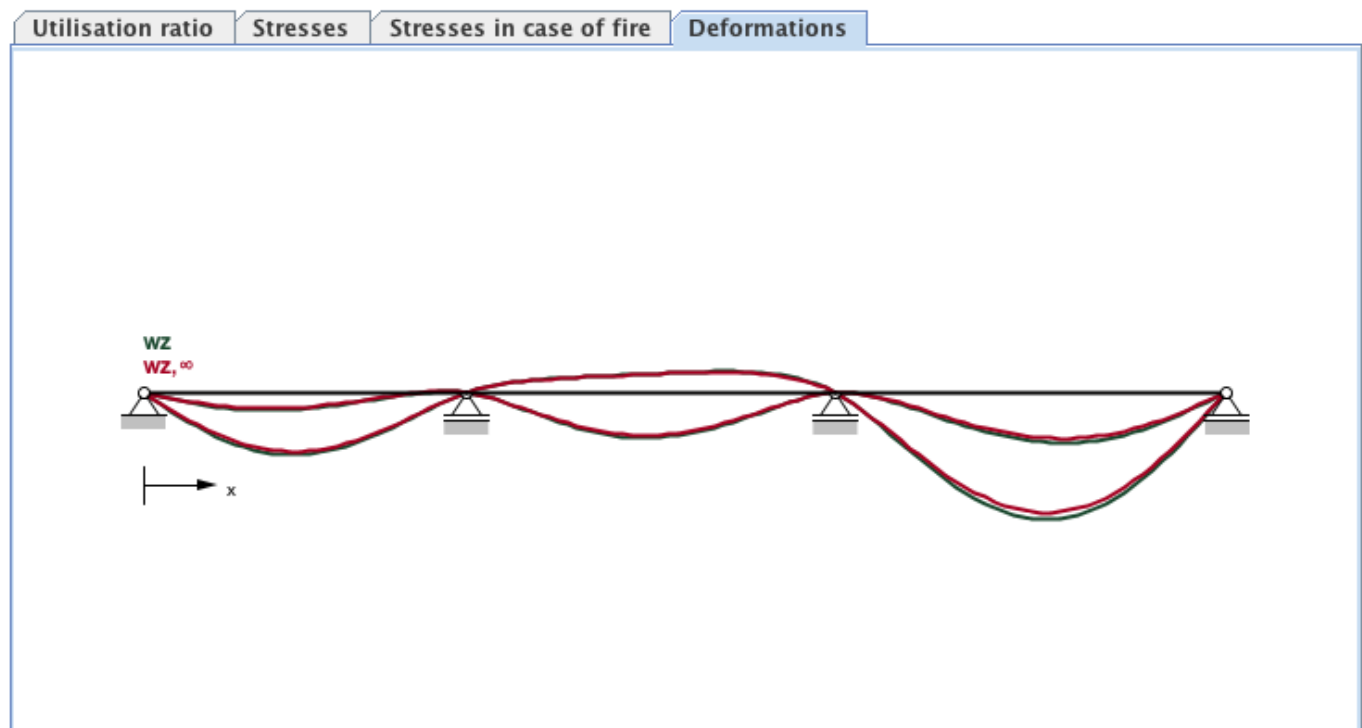
The tab "Utilisation" shows the distribution of the governing utilisation ratios along the beam. Areas in which the results may differ from the exact solution are marked here.



The tab "Stresses" shows the governing stresses resulting from the ULS verification. If a structural fire design was carried out, the governing stresses in case of fire are shown in the tab "Stresses in case of fire"

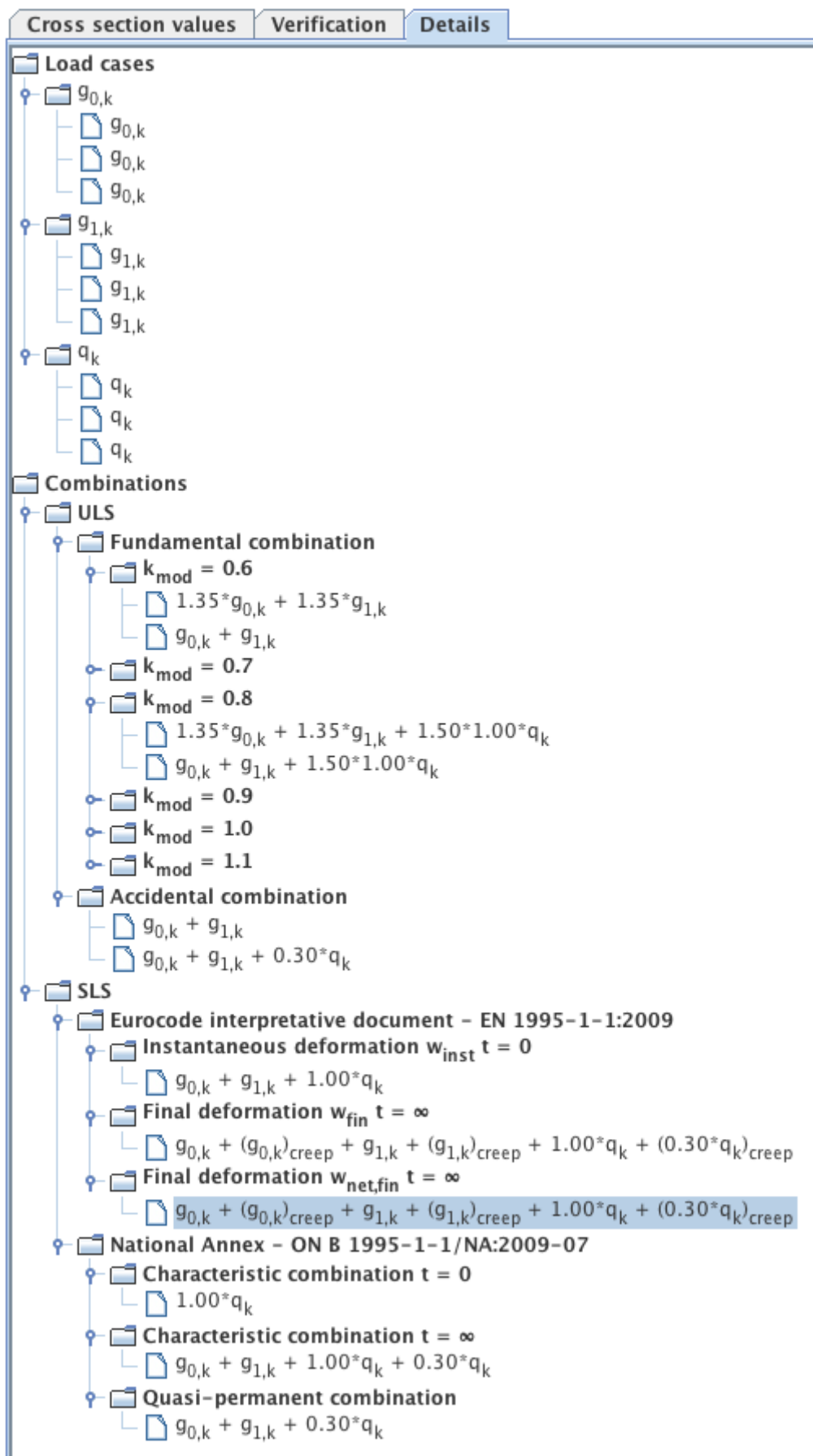


The tab "Deformations" shows the deformed system or the envelope given by the minimum and maximum deformation resulting from the governing SLS verification.



Detailed results

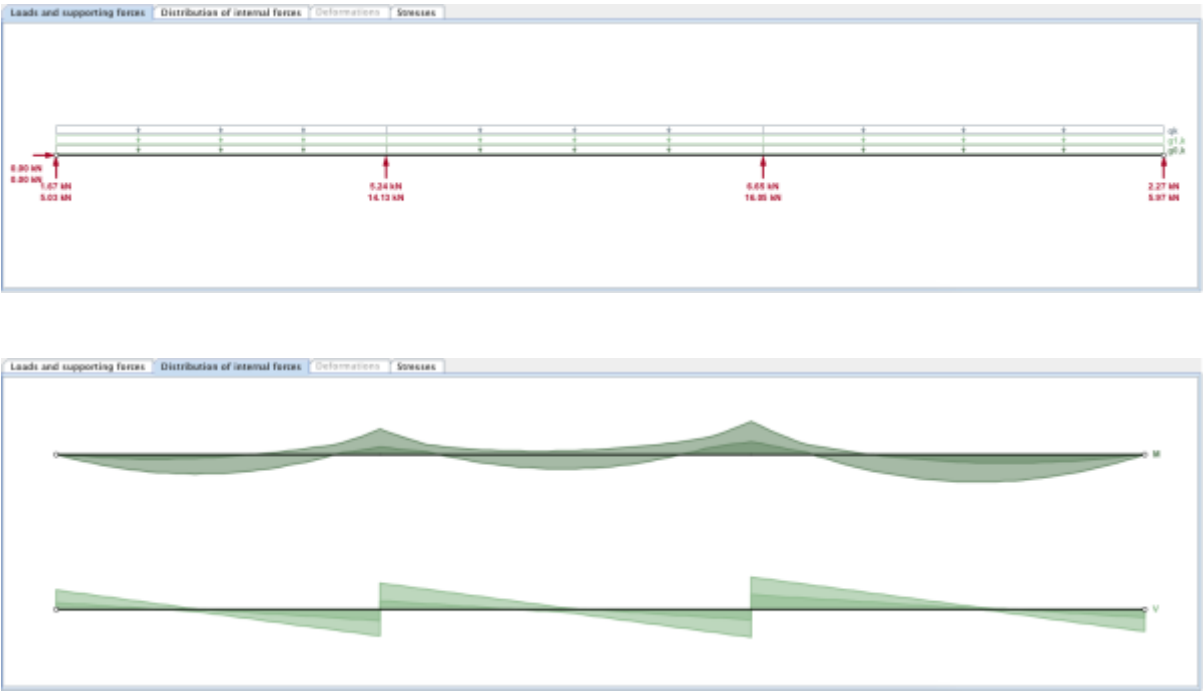
The detailed results can be retrieved in the tab "Details". The "tree" on the left side offers the possibility to choose the respective load case or combination.

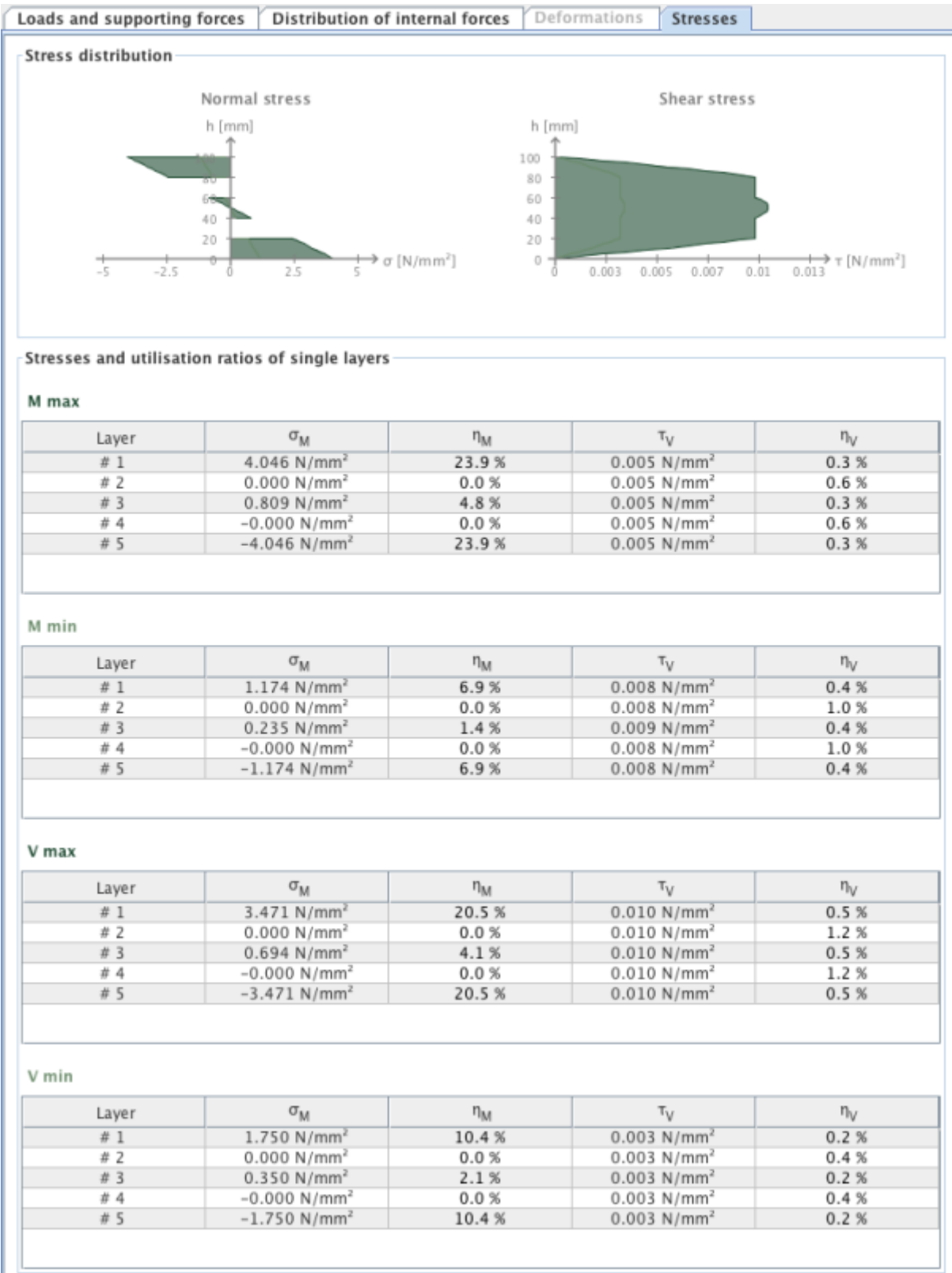


The results of this choice (internal forces, deformations) are then shown for each of the calculation sections of each field (number depending on the information in the settings) in the table on the top right.

Field	x	M _{max}	M	V	M _{min}	M	V	V _{max}	M	V	V _{min}	M	V
1	0.0 m	-0.00 kNm	1.87 kNm		-0.00 kNm	5.03 kNm		-0.00 kNm	5.03 kNm		-0.00 kNm	1.87 kNm	
1	0.1 m	0.49 kNm	4.70 kNm		0.38 kNm	1.51 kNm		0.49 kNm	4.70 kNm		0.38 kNm	1.51 kNm	
1	0.2 m	0.94 kNm	4.37 kNm		0.30 kNm	1.98 kNm		0.94 kNm	4.37 kNm		0.30 kNm	1.98 kNm	
1	0.3 m	1.36 kNm	4.03 kNm		0.41 kNm	1.21 kNm		1.36 kNm	4.03 kNm		0.41 kNm	1.21 kNm	
1	0.4 m	1.75 kNm	3.70 kNm		0.54 kNm	1.05 kNm		1.75 kNm	3.70 kNm		0.54 kNm	1.05 kNm	
1	0.5 m	2.10 kNm	3.37 kNm		0.64 kNm	0.90 kNm		2.10 kNm	3.37 kNm		0.64 kNm	0.90 kNm	
1	0.6 m	2.42 kNm	3.04 kNm		0.72 kNm	0.75 kNm		2.42 kNm	3.04 kNm		0.72 kNm	0.75 kNm	
1	0.7 m	2.71 kNm	2.70 kNm		0.79 kNm	0.60 kNm		2.71 kNm	2.70 kNm		0.79 kNm	0.60 kNm	
1	0.8 m	2.86 kNm	2.37 kNm		0.84 kNm	0.44 kNm		2.86 kNm	2.37 kNm		0.84 kNm	0.44 kNm	
1	0.9 m	3.16 kNm	2.04 kNm		0.88 kNm	0.29 kNm		3.16 kNm	2.04 kNm		0.88 kNm	0.29 kNm	
1	1.0 m	3.37 kNm	1.71 kNm		0.90 kNm	0.14 kNm		3.37 kNm	1.71 kNm		0.90 kNm	0.14 kNm	
1	1.1 m	5.52 kNm	1.37 kNm		0.91 kNm	-0.01 kNm		5.52 kNm	1.37 kNm		0.91 kNm	-0.01 kNm	
1	1.2 m	5.84 kNm	1.04 kNm		0.90 kNm	-0.17 kNm		5.84 kNm	1.04 kNm		0.90 kNm	-0.17 kNm	
1	1.3 m	5.73 kNm	0.71 kNm		0.88 kNm	-0.32 kNm		5.73 kNm	0.71 kNm		0.88 kNm	-0.32 kNm	
1	1.4 m	5.79 kNm	0.38 kNm		0.84 kNm	-0.47 kNm		5.79 kNm	0.38 kNm		0.84 kNm	-0.47 kNm	
1	1.5 m	5.81 kNm	0.04 kNm		0.78 kNm	-0.62 kNm		5.81 kNm	0.04 kNm		0.78 kNm	-0.62 kNm	
1	1.6 m	5.79 kNm	-0.29 kNm		0.71 kNm	-0.78 kNm		5.79 kNm	-0.29 kNm		0.71 kNm	-0.78 kNm	
1	1.7 m	5.75 kNm	-0.62 kNm		0.63 kNm	-0.93 kNm		5.75 kNm	-0.62 kNm		0.63 kNm	-0.93 kNm	
1	1.8 m	5.67 kNm	-0.95 kNm		0.53 kNm	-1.08 kNm		5.67 kNm	-0.95 kNm		0.53 kNm	-1.08 kNm	
1	1.9 m	5.56 kNm	-1.29 kNm		0.41 kNm	-1.23 kNm		5.56 kNm	-1.29 kNm		0.41 kNm	-1.23 kNm	
1	2.0 m	5.41 kNm	-1.62 kNm		0.28 kNm	-1.39 kNm		5.41 kNm	-1.62 kNm		0.28 kNm	-1.39 kNm	
1	2.1 m	5.21 kNm	-1.95 kNm		0.11 kNm	-1.54 kNm		5.21 kNm	-1.95 kNm		0.11 kNm	-1.54 kNm	
1	2.2 m	5.02 kNm	-2.28 kNm		-0.01 kNm	-1.69 kNm		5.02 kNm	-2.28 kNm		-0.01 kNm	-1.69 kNm	
1	2.3 m	2.78 kNm	-2.62 kNm		-0.21 kNm	-1.84 kNm		2.78 kNm	-2.62 kNm		-0.21 kNm	-1.84 kNm	
1	2.4 m	2.50 kNm	-2.95 kNm		-0.40 kNm	-1.99 kNm		2.50 kNm	-2.95 kNm		-0.40 kNm	-1.99 kNm	
1	2.5 m	2.19 kNm	-3.28 kNm		-0.60 kNm	-2.15 kNm		2.19 kNm	-3.28 kNm		-0.60 kNm	-2.15 kNm	
1	2.6 m	1.84 kNm	-3.61 kNm		-0.83 kNm	-2.30 kNm		1.84 kNm	-3.61 kNm		-0.83 kNm	-2.30 kNm	
1	2.7 m	1.46 kNm	-3.95 kNm		-1.06 kNm	-2.45 kNm		1.46 kNm	-3.95 kNm		-1.06 kNm	-2.45 kNm	
1	2.8 m	1.05 kNm	-4.28 kNm		-1.32 kNm	-2.61 kNm		1.05 kNm	-4.28 kNm		-1.32 kNm	-2.61 kNm	
1	2.9 m	0.61 kNm	-4.61 kNm		-1.59 kNm	-2.76 kNm		0.61 kNm	-4.61 kNm		-1.59 kNm	-2.76 kNm	
1	3.0 m	0.13 kNm	-4.94 kNm		-1.87 kNm	-2.91 kNm		0.13 kNm	-4.94 kNm		-1.87 kNm	-2.91 kNm	
1	3.1 m	-0.10 kNm	-2.46 kNm		-2.25 kNm	-5.88 kNm		-0.10 kNm	-2.46 kNm		-2.25 kNm	-5.88 kNm	
1	3.2 m	-0.56 kNm	-2.61 kNm		-2.85 kNm	-6.21 kNm		-0.56 kNm	-2.61 kNm		-2.85 kNm	-6.21 kNm	
1	3.3 m	-0.81 kNm	-2.77 kNm		-3.40 kNm	-6.54 kNm		-0.81 kNm	-2.77 kNm		-3.40 kNm	-6.54 kNm	
1	3.4 m	-1.11 kNm	-2.92 kNm		-3.86 kNm	-6.88 kNm		-1.11 kNm	-2.92 kNm		-3.86 kNm	-6.88 kNm	
1	3.5 m	-1.41 kNm	-3.07 kNm		-4.86 kNm	-7.21 kNm		-1.41 kNm	-3.07 kNm		-4.86 kNm	-7.21 kNm	
2	3.5 m	-1.41 kNm	2.18 kNm		-4.86 kNm	6.92 kNm		-4.86 kNm	6.92 kNm		-1.41 kNm	2.18 kNm	
2	3.6 m	-1.10 kNm	2.01 kNm		-4.19 kNm	6.58 kNm		-4.19 kNm	6.58 kNm		-1.10 kNm	2.01 kNm	
2	3.7 m	-1.01 kNm	1.88 kNm		-3.55 kNm	6.25 kNm		-3.55 kNm	6.25 kNm		-1.01 kNm	1.88 kNm	
2	3.8 m	-0.83 kNm	1.71 kNm		-2.94 kNm	5.92 kNm		-2.94 kNm	5.92 kNm		-0.83 kNm	1.71 kNm	
2	3.9 m	-0.87 kNm	1.55 kNm		-2.36 kNm	5.59 kNm		-2.36 kNm	5.59 kNm		-0.87 kNm	1.55 kNm	
2	4.0 m	-0.46 kNm	4.15 kNm		-1.88 kNm	2.51 kNm		-1.82 kNm	5.25 kNm		-0.52 kNm	1.40 kNm	
2	4.1 m	-0.07 kNm	3.81 kNm		-1.63 kNm	2.36 kNm		-1.11 kNm	4.92 kNm		-0.39 kNm	1.25 kNm	
2	4.2 m	0.10 kNm	3.48 kNm		-1.40 kNm	2.20 kNm		-0.84 kNm	4.59 kNm		-0.27 kNm	1.10 kNm	

By choosing the desired calculation section in the table, the loads and supporting forces, the distribution of internal forces and the deformations as well as the calculated stresses are shown under different tabs on the bottom right.





Module "CLT-Plate 1D - Continuous beam" · 2017/11/14 17:11

Design loads and design factors

The loads entered, are at design level.

The following loads can be applied:

- vertical load q_d
- horizontal load H_d
- load $q_{\perp,d}$ perpendicular to the wall

Design loads		Design factors	
q_d	156 $\frac{\text{kN}}{\text{m}}$ ⚙️	k_{mod}	0.9
Percentage of g_d	50 %	ξ	0.85
H_d	36 kN	Δy_{Hd}	0 m
$q_{\perp,d}$	0.9 $\frac{\text{kN}}{\text{m}^2}$	k_{sys}	1.1 ⓘ

Should it be necessary to define an eccentric load application of q_d , the input of e_{qd} can be displayed by pressing the button ⚙️.

e_{qd} 0 m

For the calculation of the tensile force in the anchor it is necessary to know the proportion of the permanent vertical loads (percentage of g_d). Since the permanent vertical load has a relieving effect on the anchor force, this is reduced by multiplication with ξ .

By default, the horizontal load is applied at top of the wall, but can be shifted by entering Δy_{Hd} .

Furthermore, the design factors are specified here.

Design loads and design factors must also be specified for the case of fire.

Fire		Design factors	
Design loads		$k_{\text{mod,fi}}$	
q_d	93.6 $\frac{\text{kN}}{\text{m}}$ ⚙️	$Y_{M,fi}$	1.00
Percentage of g_d	100 %		
H_d	0 kN		
$q_{\perp,d}$	0 $\frac{\text{kN}}{\text{m}^2}$		
	ξ 0.85		
	Δy_{Hd} 0 m		

Stability

For the verification of stability, the buckling length coefficients β_{lk} and in case of fire $\beta_{lk,fi}$ have to be defined. The buckling length is then calculated from the buckling length coefficient and the wall height. The buckling coefficient k_c , which is required for the verification, is then automatically determined from the buckling length and the specific cross-section.

Stability

β_{ek}

ℓ_k

$\beta_{ek,fi}$

$\ell_{k,fi}$

β_c

k_{05}

k_c

$k_{c,fi}$

The straightness factor β_c and the conversion factor to the 5% quantile k_{05} can be changed in the settings.

Connection wall floor

For the connection in the floor joint, 4 calculation models are currently available:

- Tension, compression discrete & shear continuously
- Compression diskret & tension, shear continuously
- Tension, compression linear & shear continuously
- Tension, compression, shear continuously

The chosen model should take into account the existing fasteners in the floor joint.

Depending on the selected calculation model, different inputs are required.

Tension, compression discrete & shear continuously

This model should be selected if tie rods and shear angles are used. Here the load widths for compression and tension are to be entered and an edge distance of the tie rod. The distance of the resulting tensile and compressive force is calculated automatically from this.

Connection wall floor

Model for H_d

Tension, compression discrete | shear continuously

Distance resultant tension / compression

m

Load width compression

m

Load width tension

m

Edge distance

m

Compression diskret & tension, shear continuously

This model assumes continuous tensile force transmission, as for example in the case of the connection with the SHERPA CLT connector. Here, only the load application width of the compression zone has to be defined.

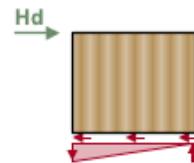
Connection wall floor

Model for H_d

Compression discrete | tension, shear continuously

Load width compression

0.1 m

**Tension, compression linear & shear continuously**

This model corresponds to the beam theory and should only be used for very narrow wall strips.

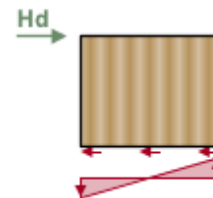
Connection wall floor

Model for H_d

Tension, compression linear | shear continuously



The selected model is not recommended for these wall dimensions!

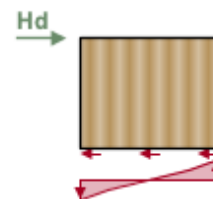
**Tension, compression, shear continuously**

This model corresponds to an approximate solution of a plate loaded in plane.

Connection wall floor

Model for H_d

Tension, compression, shear continuously

**Spring model**

In this model, tensile and shear spring stiffnesses of discretely arranged fasteners can be entered as well as a distribution width for the respective stress direction. The contact (compressive stress) is taken into account by means of an elastic bedding (input of the bedding factor c in kN/m/m).

Connection wall floor

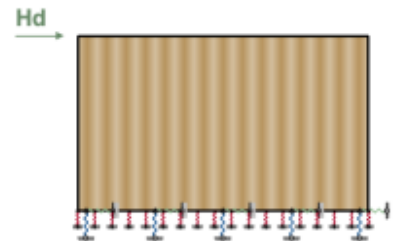
Model for H_d **Spring model**

Number	x	c_{tension}	c_{shear}	w_{tension}	w_{shear}
1	0.15 m	9,500 kN/m	3,300 kN/m	0.1 m	0.1 m
2	1.325 m	9,500 kN/m	3,300 kN/m	0.1 m	0.1 m
3	2.5 m	9,500 kN/m	3,300 kN/m	0.1 m	0.1 m
4	3.675 m	9,500 kN/m	3,300 kN/m	0.1 m	0.1 m
5	4.85 m	9,500 kN/m	3,300 kN/m	0.1 m	0.1 m



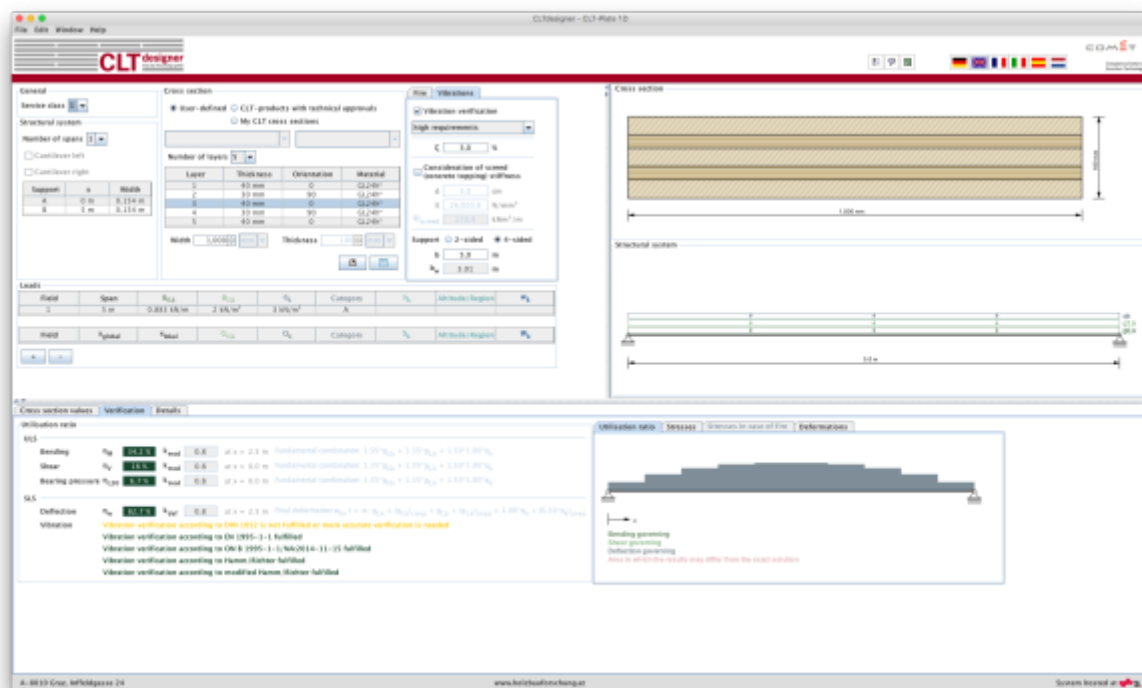
Elastic bedding

c kN/m²



Fire

See Module "CLT-Plate 1D - Continuous beam"



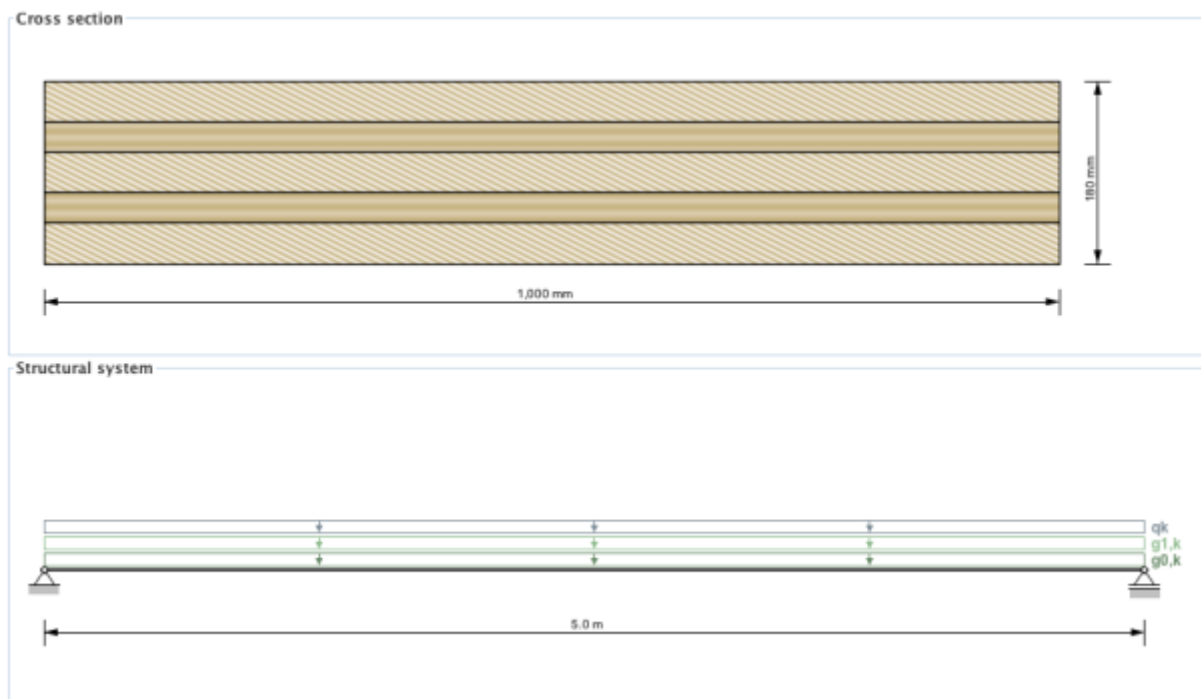
Input data

The input is divided into:

- general information about the project and the considered structural element
- definitions of the structural system
- definitions of the cross section
- input of the loads

- informations about fire and vibration parameters

A graphical representation of the input data is shown on the right side. This offers the possibility for a fast check of the input data.



General

The input field "General" defines the service class. It is only allowed to use CLT elements in areas of service class 1 and 2.

- Service class 1 (interior service condition) is in general consistent with a common utilisation of living spaces.
- Service class 2 (protected exterior service condition) is generally used for open but roofed structures.

General

Service class

Structural system

In the current version a continuous beam with a maximum of 7 spans including a cantilever on the left and right side can be analysed. The supporting width and span of field (via x-value in the table) can be defined within this input field.

Structural system

Number of spans 1

☐ Cantilever left

☐ Cantilever right

Support	x	Width
A	0 m	0.154 m
B	5 m	0.154 m

Cross section

The cross section can be defined by the user or by choosing a typical cross section of a proprietary CLT product. There is also the possibility to save own CLT cross sections in a library. The elements are subdivided by the number of layers.

If a user-defined cross section is entered, the thickness and orientation of each layer can be changed. Furthermore, the material can be changed for all layers. The thickness of each layer has to be within the range of 6.0 mm to 45 mm. In the case of proprietary CLT products, the strength class of lumber and the orientation can be changed. If the orientation is changed, the whole cross section is rotated.

Cross section

☒ User-defined
☐ CLT-products with technical approvals
☐ My CLT cross sections

Number of layers 5

Layer	Thickness	Orientation	Material
1	40 mm	0	GL24h*
2	20 mm	90	GL24h*
3	40 mm	0	GL24h*
4	20 mm	90	GL24h*
5	40 mm	0	GL24h*

Width 1,000 mm
Thickness 160 mm


Ratio board thickness / width t/a 1:4


Beta! Optimise cross section...

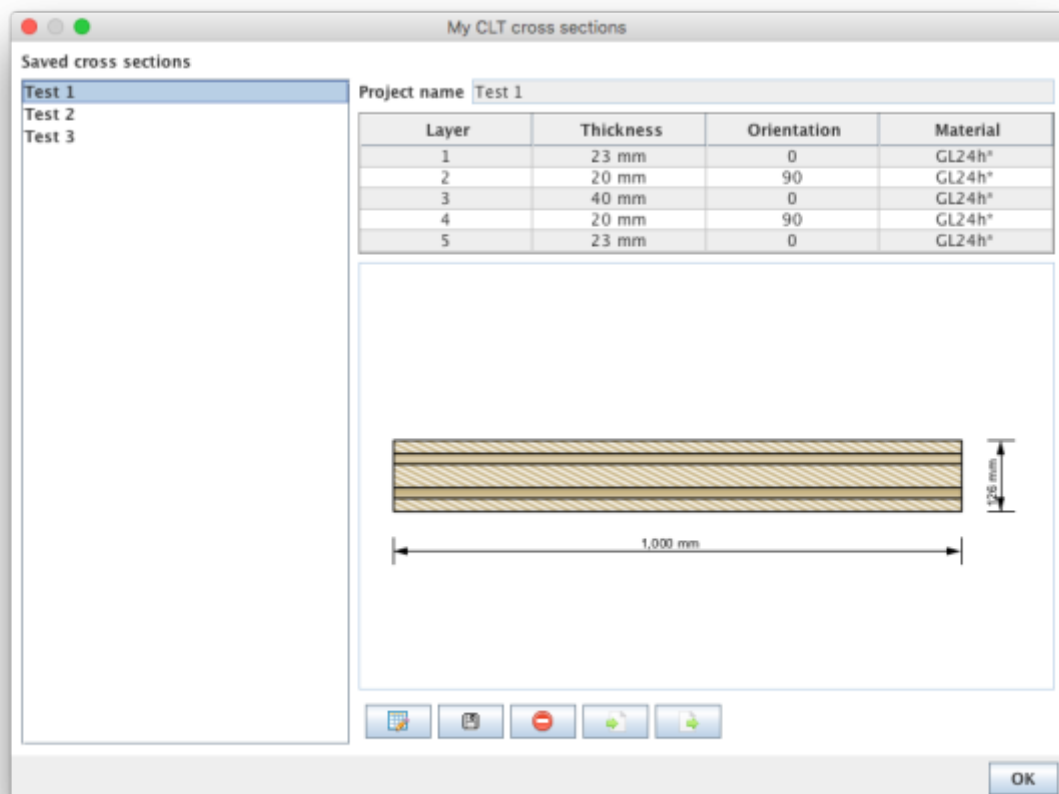
The width of the CLT plate strips can be also defined in this field. The default value is set to 1 m. The thickness of the CLT plate is calculated automatically based on the thickness of the single layers.






The ratio of board thickness to board width can also be changed here. The default setting is 1:4.

My CLT cross sections

By clicking the button  the current cross section can be stored in the library and be retrieved by selecting "My CLT cross sections" later on.

The library can be displayed with the button .



- The edit mode can be entered by clicking on . Currently, only the name of the stored cross section can be changed.
- With  the changes are saved.
- With  the chosen cross section in the sidebar can be removed from the library.
- With  cross sections from a csv file can be imported.
- With  the cross sections from the library can be exported to a csv file.



Syntax of the csv file

name;number of layers n ;layer thickness in [m] t_1 to t_n ;orientation of the layers o_1 to o_n (0


or 90);name of material

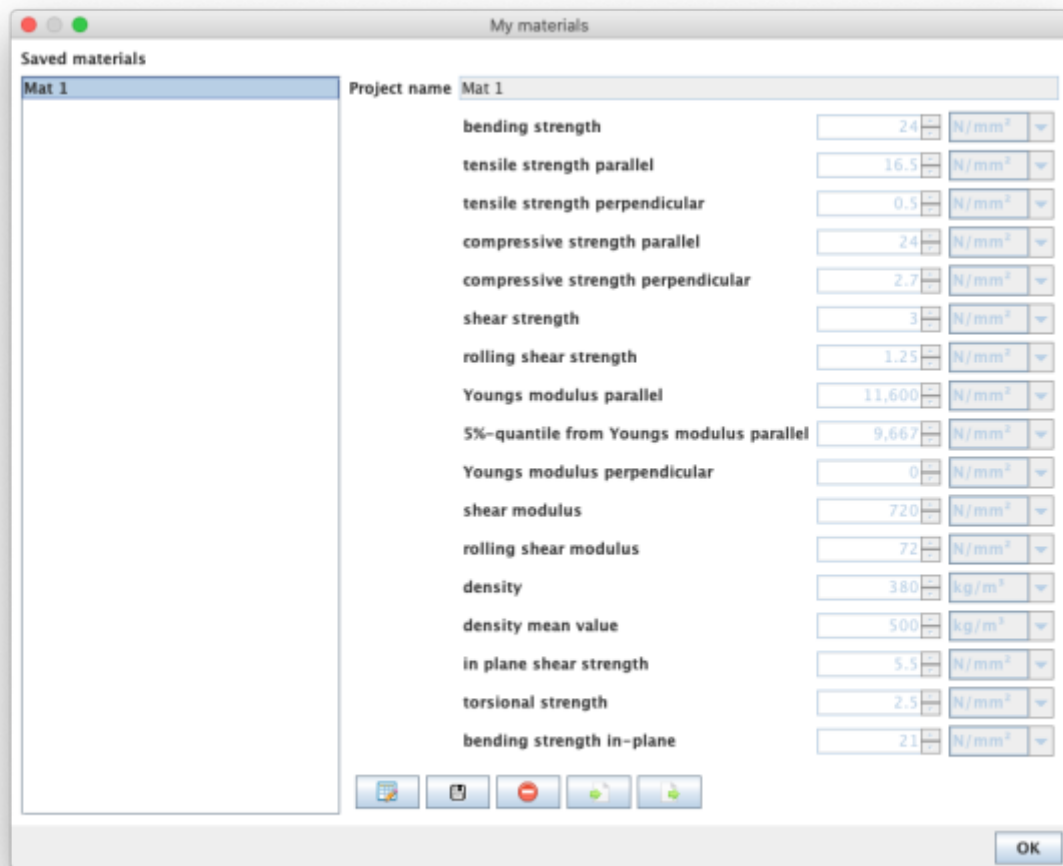


Example:






Test layup;5;0.03;0.02;0.02;0.02;0.03;90;0;90;0;90;GL24h*

My materials

With the button  the material library can be displayed.



Property	Value	Unit
bending strength	24	N/mm ²
tensile strength parallel	16.5	N/mm ²
tensile strength perpendicular	0.5	N/mm ²
compressive strength parallel	24	N/mm ²
compressive strength perpendicular	2.7	N/mm ²
shear strength	3	N/mm ²
rolling shear strength	1.25	N/mm ²
Youngs modulus parallel	11,600	N/mm ²
5%-quantile from Youngs modulus parallel	9,667	N/mm ²
Youngs modulus perpendicular	0	N/mm ²
shear modulus	720	N/mm ²
rolling shear modulus	72	N/mm ²
density	380	kg/m ³
density mean value	500	kg/m ³
in plane shear strength	5.5	N/mm ²
torsional strength	2.5	N/mm ²
bending strength in-plane	21	N/mm ²

- With  the edit mode can be entered.
- With  the changes are saved.
- With  the chosen material in the sidebar can be removed from the library.
- With  materials from a csv file can be imported.
- With  the materials from the library can be exported to a csv file.



Syntax of the csv file

1. row: description of the parameters

2. row: units of the parameters

3. row: value

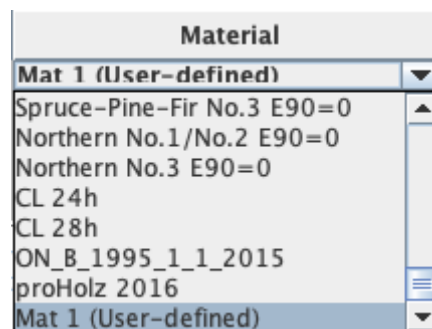
delimiter: ";"




Example:

Name;f_m,k;f_t,0,k;f_t,90,k;f_c,k;f_c,90,k;f_v,k;f_r,k;E_0;E_0,05;E_90;G;G_r;rho_k;rho_mean;f_v,k,IP;f_T,k;f_m,k,IP
;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;N/mm2;kg/m3;kg/m3;N/mm2;N/mm2;N/mm2
Mat 1;24;16.5;0.5;24;2.7;3;1.25;11600;9667;0;720;72;380;500;5.5;2.5;21

The user-defined materials are then displayed in the material selection list.



Optimization of layup

Use the button  to display the window for layup optimization.

Optimisation

Consider in the optimisation:

Producer

☐ best wood SCHNEIDER ☒ KLH

☐ Cross Timber Systems ☐ Mayr-Melnhof Holz

☐ Derix ☐ Piveteaubois

☐ Eugen Decker ☐ Stora Enso

☐ Hasslacher

Number of layers

☐ 3 ☒ 5 ☐ 6 ☒ 7 ☐ 8 ☐ 9 ☐ 11

Plate thickness

min mm max mm

Saved cross sections

☐ My CLT cross sections

Options

☐ Outer cross layers ☒ Double layers

☐ Vibration verification according to EN

Start **Stop**

Producer	Cross section	Plate thickness	Governing proof	Utilisation ratio
KLH	180mm 5s DL	180 mm	Vibration	99.2 %
KLH	190mm 5s DL	190 mm	Vibration	95.4 %
KLH	200mm 5s DL	200 mm	Vibration	90.1 %
KLH	220mm 7s DL	220 mm	Vibration	85.3 %
KLH	240mm 7s DL	240 mm	Vibration	78.4 %
KLH	180mm 7ss DL	180 mm	Vibration	93.9 %
KLH	200mm 7ss DL	200 mm	Vibration	82.8 %
KLH	220mm 7ss DL	220 mm	Vibration	73.1 %
KLH	240mm 7ss DL	240 mm	Vibration	65.8 %
KLH	260mm 7ss DL	260 mm	Vibration	61.6 %
KLH	280mm 7ss DL	280 mm	Vibration	58.2 %

Choose the selected cross section

With the help of this tool, the possible layouts can be determined for the given system and load situation. The optimization can be restricted with regard to producers, number of layers or by means of limits for the panel thickness. Furthermore, outer cross layers or double layers can be included or excluded. With the option "Vibration verification according to EN" the base document is included in the vibration check or not.

With the buttons "Start" and "Stop" the calculation is controlled. Please be patient, depending on the selected parameter the calculation may take a little longer.

The possible setups are then displayed in the table and the selected setup can be transferred to the main window by clicking the "Choose the selected cross section" button.

Loads

The loads are divided into the dead load (weight of the plate) ($g_{0,k}$), permanent loads ($g_{1,k}$), imposed load (q_k), snow load (s_k) and wind load (w_k). This classification is necessary to automatically carry out calculations for different load case combinations.

The plate weight is calculated automatically. The calculation method can be selected in the settings/preferences window. The default calculation method is in accordance with ON B 1991-1-1. A unit weight of 5.5 kN/m^3 is assumed in the calculation. However, the unit weight may also be

calculated using:

- calculation based on the mean value of density of the chosen material
- calculation based on a user-defined density

When entering the imposed loads, one of the following categories has to be chosen:

- A: Areas for domestic and residential activities
- B: Office areas
- C: Areas where people may congregate (with the exception of areas defined under category A, B and D)
- D: Shopping areas
- E: Areas for storage and industrial activities
- F: Traffic and parking areas for light-duty vehicles
- G: Traffic and parking areas for medium-duty vehicles
- H: Roofs

When entering the snow load, the country code or an altitude above sea level where the structure will be located has to be specified:

- < 1000 m
- > 1000 m
- FIN, IS, N, S

The span of each field can also be modified in the table of distributed loads.

Concentrated loads can be entered in the second table. The position can be defined whether by the local or global x-coordinate.

Loads								
Field	Span	$g_{0,k}$	$g_{1,k}$	q_k	Category	s_k	Altitude/Region	w_k
1	3.5 m	0.55 kN/m	0.58 kN/m ²	1.2 kN/m ²	A			
2	4 m	0.55 kN/m	0.58 kN/m ²	1.2 kN/m ²	A			
3	4.25 m	0.55 kN/m	0.58 kN/m ²	1.2 kN/m ²	A			

Field	x_{global}	x_{local}	$G_{1,k}$	Q_k	Category	s_k	Altitude/Region	w_k

Fire

By choosing "Fire above" and/or "Fire below" in the tab "Fire" a structural fire design has to be carried out. The "Fire duration" is specified in minutes and can be increased (or decreased) by increments of 30 minutes by pressing the up (or down) arrows, or defined by entering a specific duration between 0 minutes and 240 minutes in the allotted box. By ticking the box next to "Fire protection system" a layer of fire protection is added to the plate. Furthermore, the position ("above" and/or "below") must be declared and the parameters t_{ch} , t_f , k_2 and k_3 must be specified. In case the failure time of the fire protection system is equal to the time until the protected component starts to burn, the option " $t_f = t_{ch}$ " shall be checked.

Fire

Vibrations

☐ Fire above

☒ Fire below

Fire duration minutes

☒ Fire protection system

☒ below

t_{ch} minutes

t_f minutes ☒ $t_f = t_{ch}$

k_2

k_3

☐ Heat resistant adhesive

☒ consider falling off of charred layers

☐ Without gaps or with bonded edges

k_{fire}

d_0 mm

Charring rate mm/min

For a user-defined cross section, options are given for specifying heat resistant adhesives, presence of grooves, and whether the layers are edge-glued. For CLT products both values are set automatically and they cannot be changed.

Some manufacturers offer CLT elements with different adhesives, so instead of selecting "Heat resistant adhesive", it is also possible to select the adhesive.

Adhesive ☒ PUR ☐ MUF

The values k_{fire} (conversion factor 20%-quantiles) and d_0 (layer thickness to take into consideration the influence of temperature exposure) are pre-set and cannot be changed. The charring rate is dependent on the option edge glued or without groove. For a user-defined cross section this value can be changed.

In some approvals or design proposals a different charring rate is expected from the 2nd layer onwards. This will show up as follows:

Charring rate from 2nd layer onwards mm/min

Vibrations

The tab "Vibrations" allows for vibration verification.

Fire
Vibrations

☒ **Vibration verification**

normal requirements ▼

ζ

3.0

 %

☒ **Consideration of screed (concrete topping) stiffness**

d

6.0

 cm
 E

26,000.0

 N/mm²
 EI_{screed}

468

 kNm²/m

Support ☐ 2-sided ☒ 4-sided

b

5.0

 m

b_w

3.13

 m

For the vibration verification the following specifications are of importance:

- high or normal requirements? This choice will have an influence on the limit values.
- modal damping factor
- consideration of the screed (concrete topping) stiffness
 - thickness of the screed (concrete topping)
 - modulus of elasticity of the screed (concrete topping)
- support (2-sided or 4-sided)
- room width b perpendicular to the load carrying direction

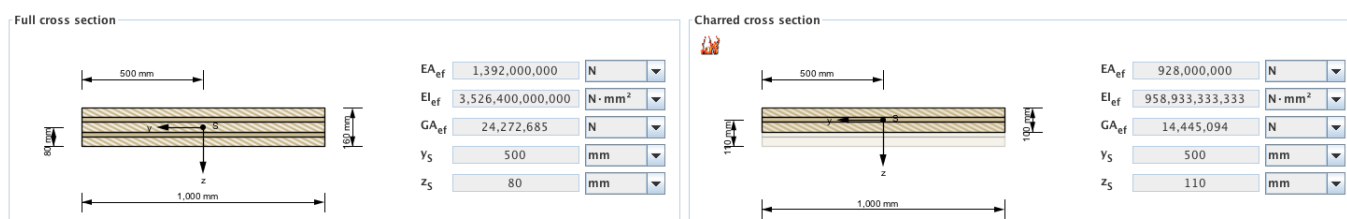
The effective width b_w of the chosen cross section used by the stiffness criteria will be specified.

Results and output

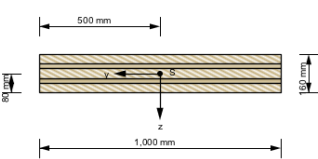
Load combinations are compiled based on the input loads entered in the "Loads" field. The respective k_{mod} - and k_{def} -values can be determined automatically based on the classification of loads (plate weight, wind load, etc.).

Cross section values

Output values generated in the tab "Cross section values" field include the effective stiffness (depending on the chosen calculation method), the position of the centre of mass for the full cross section and also for the charred cross section in case of structural fire design.



Full cross section



EA

1,392,000,000

N

$EI_{yy,A}$

185,600,000,000

N·mm²

$EI_{yy,B}$

3,340,800,000,000

N·mm²

EI_{yy}

3,526,400,000,000

N·mm²

$GA_{z,A}$

∞

N

$GA_{z,B}$

21,600,000

N

GA_z

21,600,000

N

y_s

500

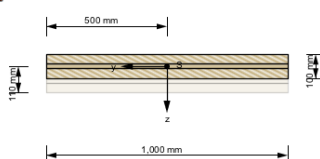
mm

z_s

80

mm

Charred cross section



EA

928,000,000

N

$EI_{yy,A}$

123,733,333,333

N·mm²

$EI_{yy,B}$

835,200,000,000

N·mm²

EI_{yy}

958,933,333,333

N·mm²

$GA_{z,A}$

∞

N

$GA_{z,B}$

10,800,000

N

GA_z

10,800,000

N

y_s

500

mm

z_s

110

mm

Summary of the results

A summary of the verifications can be retrieved via the tab "Verifications". The utilisation ratios for various limit states are colour-coded indicating if the verification is fulfilled (green), not fulfilled (red) or a more accurate verification is needed (yellow). The locations of the maximum utilisation ratio and the governing combinations are compiled in the same way.

Cross section values

Verification

Details

Utilisation ratio

ULS

Bending

η_M

28.3 %

k_{mod}

0.8

at x = 7.5 m

Fundamental combination: $1.35 \cdot g_{0,k} + 1.35 \cdot g_{1,k} + 1.50 \cdot 1.00 \cdot q_k$

Shear

η_V

13 %

k_{mod}

0.8

at x = 7.5 m

Fundamental combination: $1.35 \cdot g_{0,k} + 1.35 \cdot g_{1,k} + 1.50 \cdot 1.00 \cdot q_k$

Bearing pressure

$\eta_{c,90}$

5 %

k_{mod}

0.8

at x = 7.5 m

Fundamental combination: $1.35 \cdot g_{0,k} + 1.35 \cdot g_{1,k} + 1.50 \cdot 1.00 \cdot q_k$

SLS

Deflection

η_w

75.8 %

k_{def}

0.85

at x = 9.83 m

Final deformation $w_{tot,fin} \ t = \infty$: $g_{0,k} + (g_{0,k})^{creep} + g_{1,k} + (g_{1,k})^{creep} + 1.00 \cdot q_k + (0.30 \cdot q_k)^{creep}$

Vibration

Vibration verification according to DIN 1052 fulfilled

Vibration verification according to EN 1995-1-1 fulfilled

Vibration verification according to ON B 1995-1-1/NA:2009-07 fulfilled

Vibration verification according to Hamm/Richter fulfilled

Vibration verification according to modified Hamm/Richter fulfilled

Utilisation ratio in case of fire

ULS

Bending

$\eta_{M,fi}$

15.7 %

k_{mod}

1.0

at x = 7.5 m

Accidental combination: $g_{0,k} + g_{1,k} + 0.30 \cdot q_k$

Shear

$\eta_{V,fi}$

6.1 %

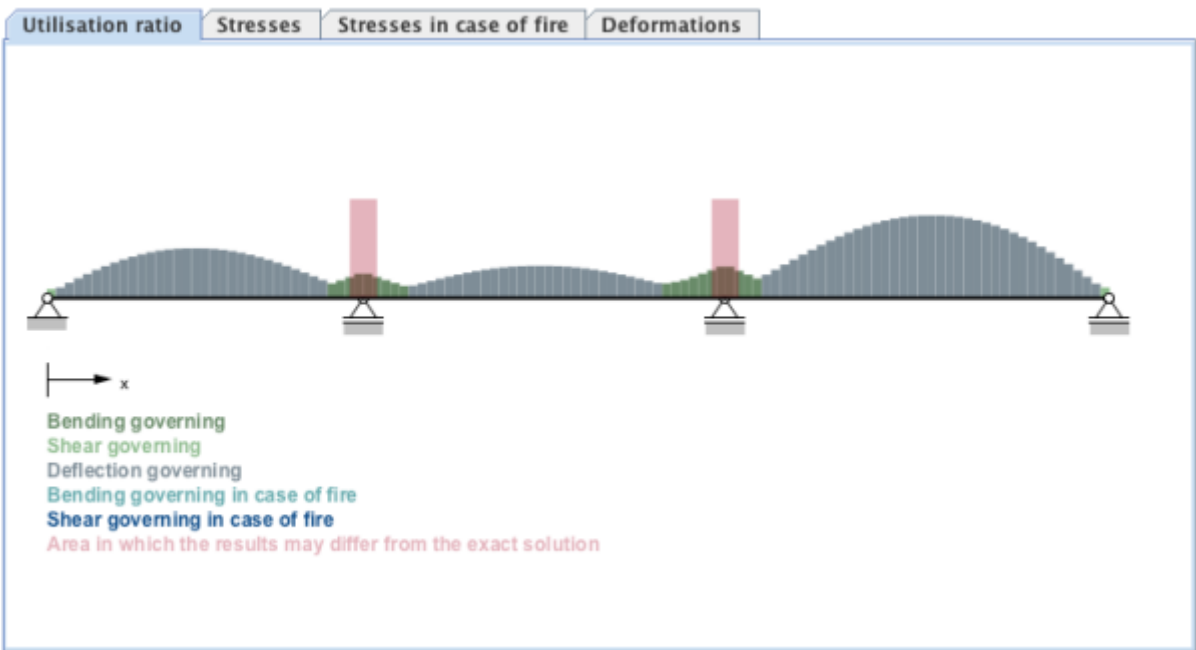
k_{mod}

1.0

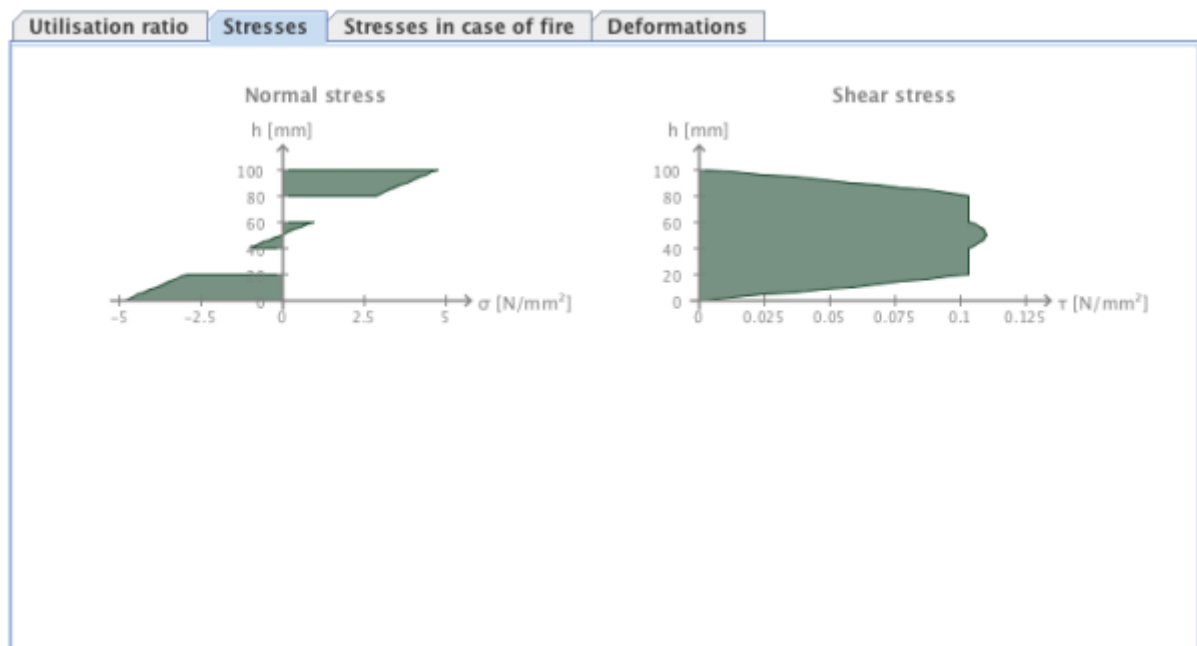
at x = 7.5 m

Accidental combination: $g_{0,k} + g_{1,k} + 0.30 \cdot q_k$

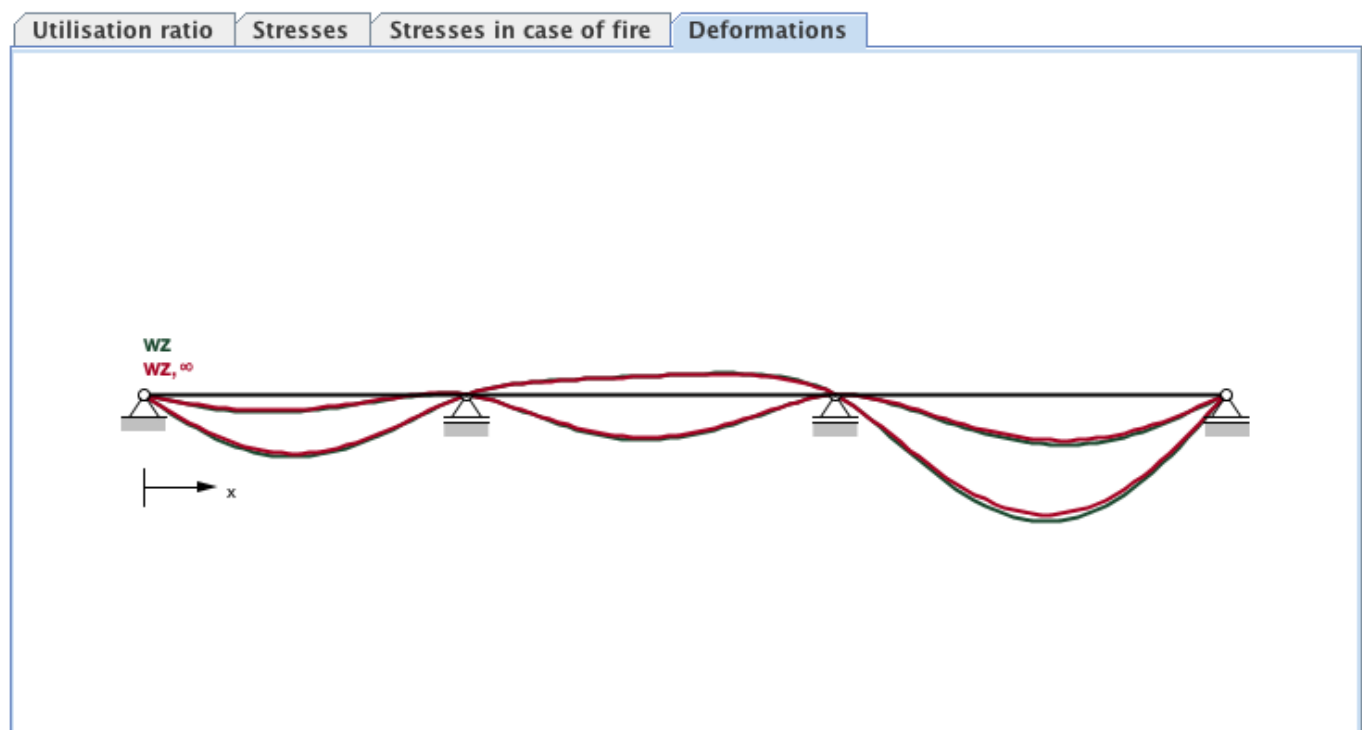
The tab "Utilisation" shows the distribution of the governing utilisation ratios along the beam. Areas in which the results may differ from the exact solution are marked here.



The tab "Stresses" shows the governing stresses resulting from the ULS verification. If a structural fire design was carried out, the governing stresses in case of fire are shown in the tab "Stresses in case of fire"

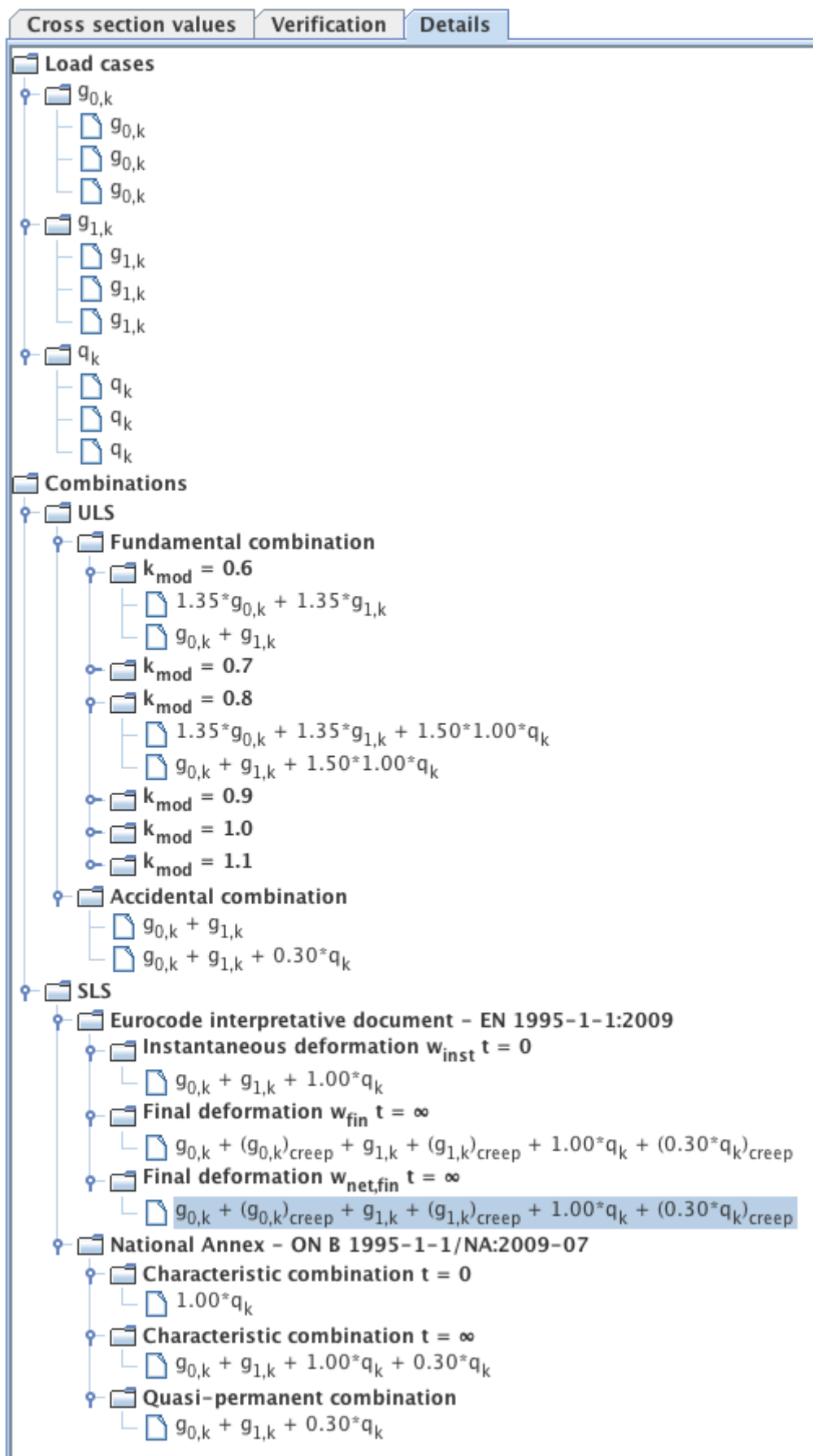


The tab "Deformations" shows the deformed system or the envelope given by the minimum and maximum deformation resulting from the governing SLS verification.



Detailed results

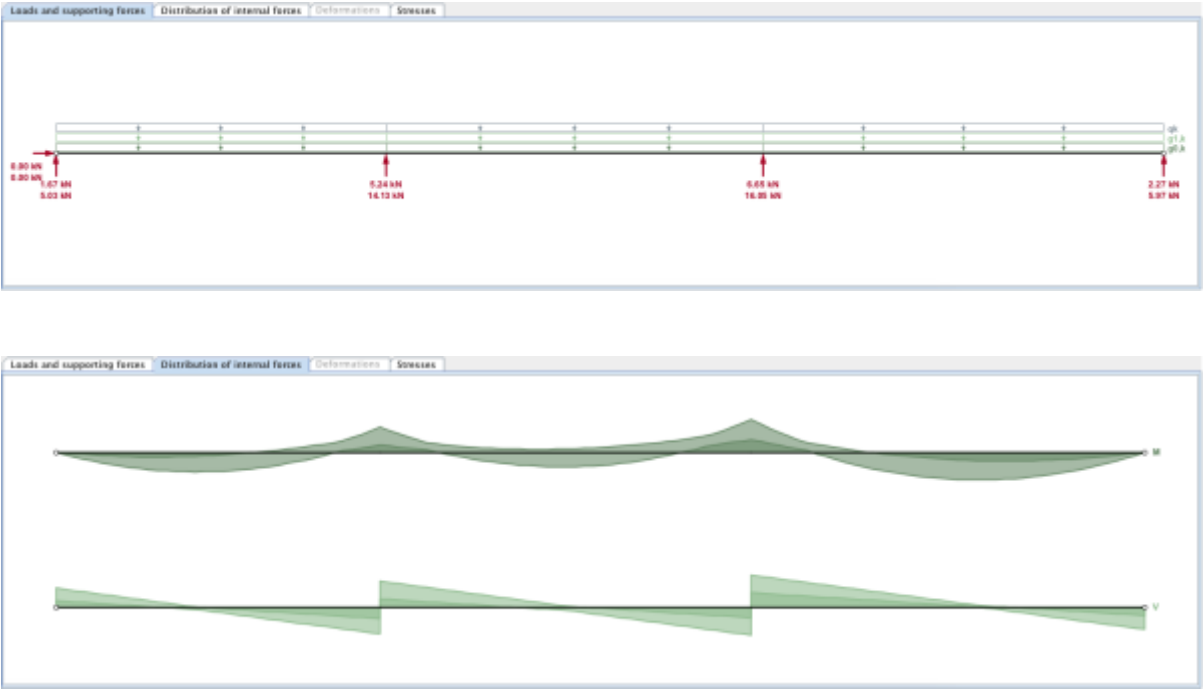
The detailed results can be retrieved in the tab "Details". The "tree" on the left side offers the possibility to choose the respective load case or combination.

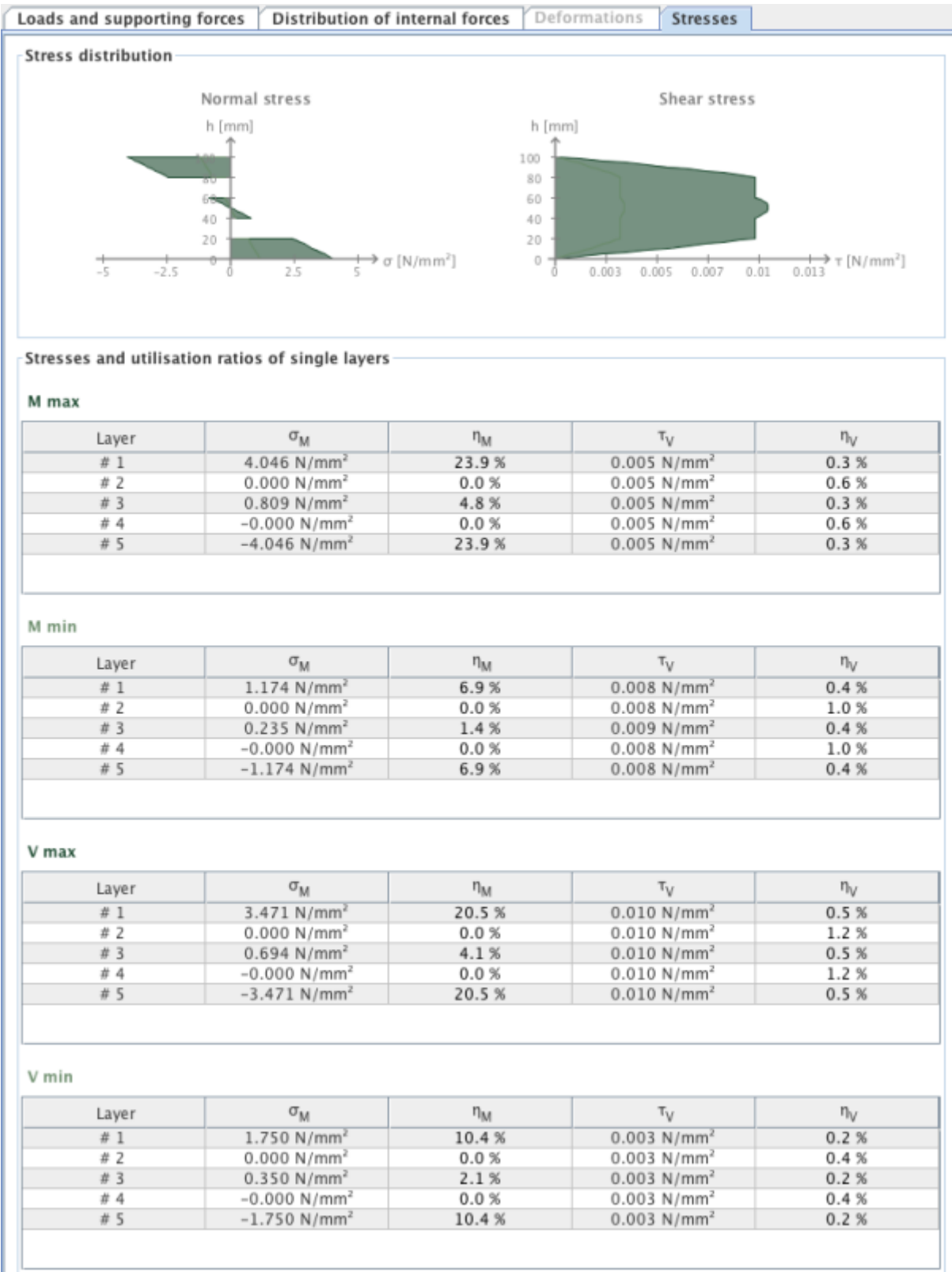


The results of this choice (internal forces, deformations) are then shown for each of the calculation sections of each field (number depending on the information in the settings) in the table on the top right.

Field	x	M _{max}	M	V	M _{min}	M	V	V _{max}	M	V	V _{min}	M	V
1	0.0 m	-0.00 kNm	1.87 kNm		-0.00 kNm	5.03 kNm		-0.00 kNm	5.03 kNm		-0.00 kNm	1.87 kNm	
1	0.1 m	0.49 kNm	4.70 kNm		0.18 kNm	1.51 kNm		0.49 kNm	4.70 kNm		0.18 kNm	1.51 kNm	
1	0.2 m	0.94 kNm	4.37 kNm		0.30 kNm	1.98 kNm		0.94 kNm	4.37 kNm		0.30 kNm	1.98 kNm	
1	0.3 m	1.38 kNm	4.03 kNm		0.41 kNm	1.21 kNm		1.38 kNm	4.03 kNm		0.41 kNm	1.21 kNm	
1	0.4 m	1.75 kNm	3.70 kNm		0.54 kNm	1.05 kNm		1.75 kNm	3.70 kNm		0.54 kNm	1.05 kNm	
1	0.5 m	2.10 kNm	3.37 kNm		0.64 kNm	0.90 kNm		2.10 kNm	3.37 kNm		0.64 kNm	0.90 kNm	
1	0.6 m	2.42 kNm	3.04 kNm		0.72 kNm	0.75 kNm		2.42 kNm	3.04 kNm		0.72 kNm	0.75 kNm	
1	0.7 m	2.71 kNm	2.70 kNm		0.79 kNm	0.60 kNm		2.71 kNm	2.70 kNm		0.79 kNm	0.60 kNm	
1	0.8 m	2.86 kNm	2.37 kNm		0.84 kNm	0.44 kNm		2.86 kNm	2.37 kNm		0.84 kNm	0.44 kNm	
1	0.9 m	3.18 kNm	2.04 kNm		0.88 kNm	0.29 kNm		3.18 kNm	2.04 kNm		0.88 kNm	0.29 kNm	
1	1.0 m	3.37 kNm	1.71 kNm		0.90 kNm	0.14 kNm		3.37 kNm	1.71 kNm		0.90 kNm	0.14 kNm	
1	1.1 m	5.52 kNm	1.37 kNm		0.91 kNm	-0.01 kNm		5.52 kNm	1.37 kNm		0.91 kNm	-0.01 kNm	
1	1.2 m	5.84 kNm	1.04 kNm		0.90 kNm	-0.17 kNm		5.84 kNm	1.04 kNm		0.90 kNm	-0.17 kNm	
1	1.3 m	5.73 kNm	0.71 kNm		0.88 kNm	-0.32 kNm		5.73 kNm	0.71 kNm		0.88 kNm	-0.32 kNm	
1	1.4 m	5.79 kNm	0.58 kNm		0.84 kNm	-0.47 kNm		5.79 kNm	0.58 kNm		0.84 kNm	-0.47 kNm	
1	1.5 m	5.81 kNm	0.04 kNm		0.78 kNm	-0.62 kNm		5.81 kNm	0.04 kNm		0.78 kNm	-0.62 kNm	
1	1.6 m	5.79 kNm	-0.29 kNm		0.71 kNm	-0.78 kNm		5.79 kNm	-0.29 kNm		0.71 kNm	-0.78 kNm	
1	1.7 m	5.75 kNm	-0.62 kNm		0.63 kNm	-0.93 kNm		5.75 kNm	-0.62 kNm		0.63 kNm	-0.93 kNm	
1	1.8 m	5.67 kNm	-0.95 kNm		0.53 kNm	-1.08 kNm		5.67 kNm	-0.95 kNm		0.53 kNm	-1.08 kNm	
1	1.9 m	5.56 kNm	-1.29 kNm		0.41 kNm	-1.23 kNm		5.56 kNm	-1.29 kNm		0.41 kNm	-1.23 kNm	
1	2.0 m	5.41 kNm	-1.62 kNm		0.28 kNm	-1.39 kNm		5.41 kNm	-1.62 kNm		0.28 kNm	-1.39 kNm	
1	2.1 m	5.21 kNm	-1.95 kNm		0.13 kNm	-1.54 kNm		5.21 kNm	-1.95 kNm		0.13 kNm	-1.54 kNm	
1	2.2 m	5.02 kNm	-2.28 kNm		-0.03 kNm	-1.69 kNm		5.02 kNm	-2.28 kNm		-0.03 kNm	-1.69 kNm	
1	2.3 m	2.78 kNm	-2.62 kNm		-0.21 kNm	-1.84 kNm		2.78 kNm	-2.62 kNm		-0.21 kNm	-1.84 kNm	
1	2.4 m	2.50 kNm	-2.95 kNm		-0.40 kNm	-1.99 kNm		2.50 kNm	-2.95 kNm		-0.40 kNm	-1.99 kNm	
1	2.5 m	2.19 kNm	-3.28 kNm		-0.60 kNm	-2.15 kNm		2.19 kNm	-3.28 kNm		-0.60 kNm	-2.15 kNm	
1	2.6 m	1.84 kNm	-3.61 kNm		-0.83 kNm	-2.30 kNm		1.84 kNm	-3.61 kNm		-0.83 kNm	-2.30 kNm	
1	2.7 m	1.46 kNm	-3.95 kNm		-1.06 kNm	-2.45 kNm		1.46 kNm	-3.95 kNm		-1.06 kNm	-2.45 kNm	
1	2.8 m	1.05 kNm	-4.28 kNm		-1.32 kNm	-2.61 kNm		1.05 kNm	-4.28 kNm		-1.32 kNm	-2.61 kNm	
1	2.9 m	0.61 kNm	-4.61 kNm		-1.59 kNm	-2.76 kNm		0.61 kNm	-4.61 kNm		-1.59 kNm	-2.76 kNm	
1	3.0 m	0.13 kNm	-4.94 kNm		-1.87 kNm	-2.91 kNm		0.13 kNm	-4.94 kNm		-1.87 kNm	-2.91 kNm	
1	3.1 m	-0.10 kNm	-2.46 kNm		-2.25 kNm	-5.88 kNm		-0.10 kNm	-2.46 kNm		-2.25 kNm	-5.88 kNm	
1	3.2 m	-0.56 kNm	-2.61 kNm		-2.85 kNm	-6.21 kNm		-0.56 kNm	-2.61 kNm		-2.85 kNm	-6.21 kNm	
1	3.3 m	-0.81 kNm	-2.77 kNm		-3.49 kNm	-6.54 kNm		-0.81 kNm	-2.77 kNm		-3.49 kNm	-6.54 kNm	
1	3.4 m	-1.11 kNm	-2.92 kNm		-4.16 kNm	-6.88 kNm		-1.11 kNm	-2.92 kNm		-4.16 kNm	-6.88 kNm	
1	3.5 m	-1.41 kNm	-3.07 kNm		-4.86 kNm	-7.21 kNm		-1.41 kNm	-3.07 kNm		-4.86 kNm	-7.21 kNm	
2	3.5 m	-1.41 kNm	2.18 kNm		-8.86 kNm	6.92 kNm		-8.86 kNm	6.92 kNm		-1.41 kNm	2.18 kNm	
2	3.6 m	-1.10 kNm	2.01 kNm		-8.19 kNm	6.58 kNm		-8.19 kNm	6.58 kNm		-1.10 kNm	2.01 kNm	
2	3.7 m	-1.03 kNm	1.88 kNm		-5.55 kNm	6.25 kNm		-5.55 kNm	6.25 kNm		-1.03 kNm	1.88 kNm	
2	3.8 m	-0.83 kNm	1.71 kNm		-2.94 kNm	5.92 kNm		-2.94 kNm	5.92 kNm		-0.83 kNm	1.71 kNm	
2	3.9 m	-0.87 kNm	1.55 kNm		-2.36 kNm	5.59 kNm		-2.36 kNm	5.59 kNm		-0.87 kNm	1.55 kNm	
2	4.0 m	-0.46 kNm	4.15 kNm		-1.88 kNm	2.51 kNm		-1.82 kNm	5.25 kNm		-0.52 kNm	1.40 kNm	
2	4.1 m	-0.07 kNm	3.81 kNm		-1.63 kNm	2.36 kNm		-1.13 kNm	4.92 kNm		-0.39 kNm	1.25 kNm	
2	4.2 m	0.10 kNm	3.48 kNm		-1.40 kNm	2.20 kNm		-0.84 kNm	4.59 kNm		-0.27 kNm	1.10 kNm	

By choosing the desired calculation section in the table, the loads and supporting forces, the distribution of internal forces and the deformations as well as the calculated stresses are shown under different tabs on the bottom right.





Module "CLT-Plate 1D - Continuous beam" · 2017/11/14 17:11

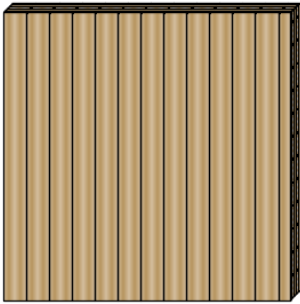
Fire left / right instead of fire above and below.

Results and output

Cross section values

The effective stiffnesses of the wall are given in the tab "cross section values" for the full cross section and in case of structural fire design for the charred cross section.

Full cross section



$K_{CLT,y}$

765,600

N·m

$S_{CLT,y}$

11,219,843

N/m

D_y

696,000,000

N/m

S_{xy}

48,580,082

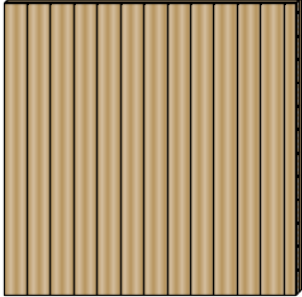
N/m

$z_{S,y}$

50

mm

Charred cross section



$K_{CLT,y}$

141,166

N·m

$S_{CLT,y}$

5,352,809

N/m

D_y

394,400,000

N/m

S_{xy}

28,053,462

N/m

$z_{S,y}$

74.76

mm

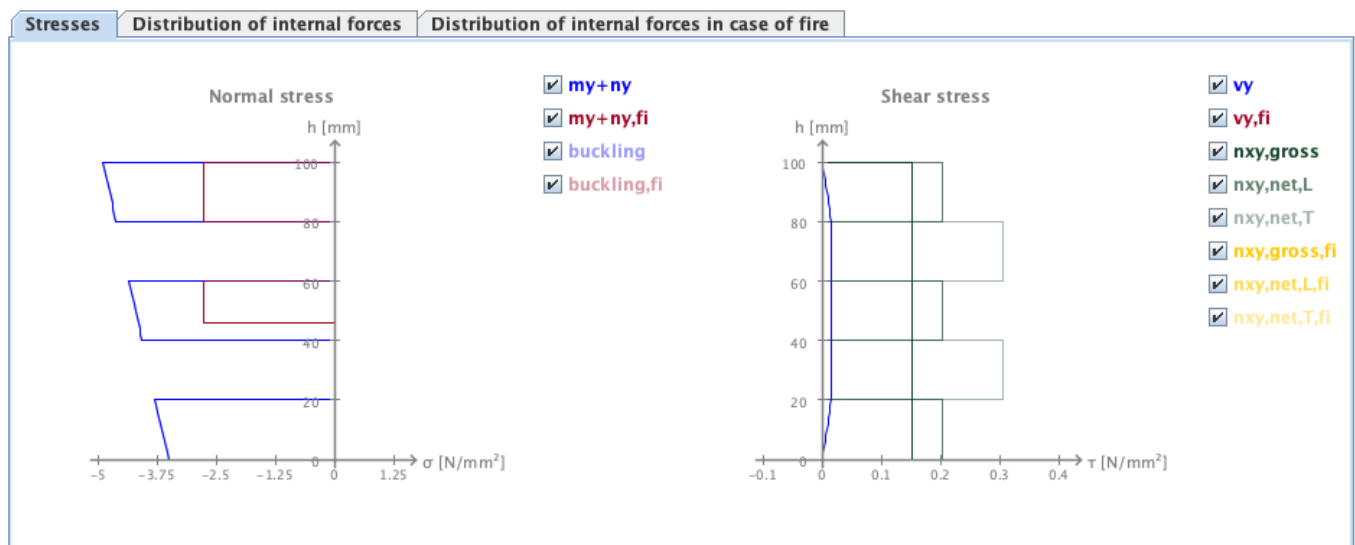
Summary of the results

In the "Verification" tab, the calculated utilisation rates for the respective design situation are displayed.

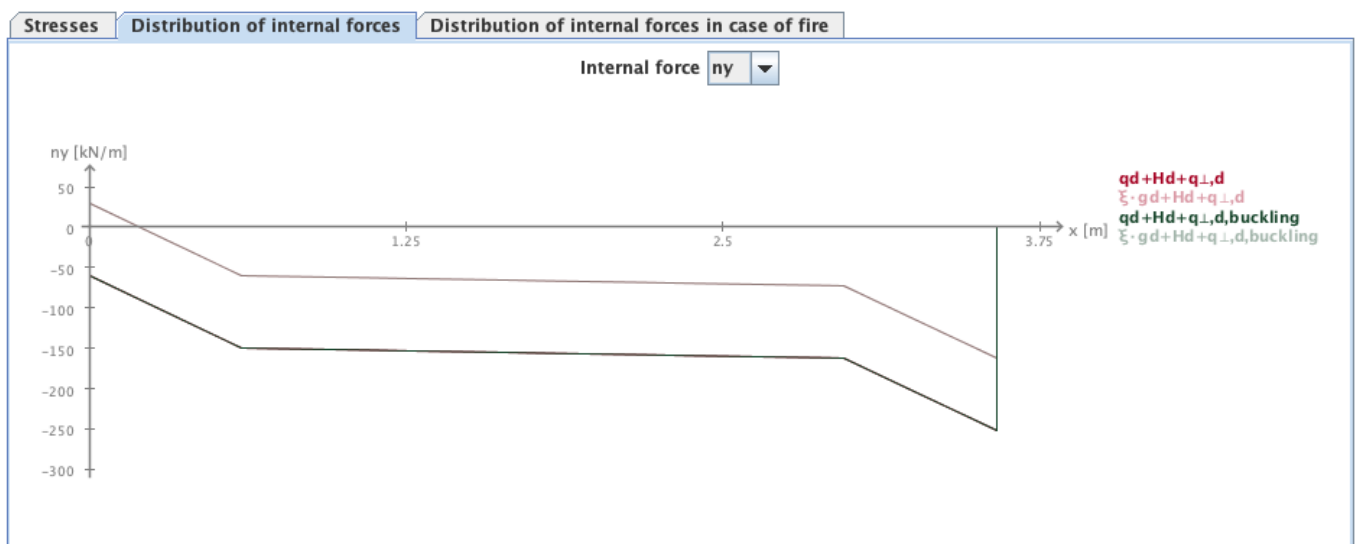
Utilisation ratios						
Bending with normal force $m_y + n_y$	$\eta_{m_y+n_y}$	24.4 %	k_{mod}	0.90	at x y = 3.58 m 1.43 m	Fundamental combination: $q_d + H_d + q_{\perp,d}$
Shear v_y	η_{v_y}	1.7 %	k_{mod}	0.90	at x y = 1.79 m 0.00 m	Fundamental combination: $q_d + H_d + q_{\perp,d}$
Shear force n_{xy} (gross shear)	$\eta_{n_{xy,gross}}$	7.1 %	k_{mod}	0.90	at x y = 1.79 m 0.00 m	Fundamental combination: $q_d + H_d + q_{\perp,d}$
Shear force n_{xy} (net shear)	$\eta_{n_{xy,net}}$	7.1 %	k_{mod}	0.90	at x y = 1.79 m 0.00 m	Fundamental combination: $q_d + H_d + q_{\perp,d}$
Shear force n_{xy} (torsion)	$\eta_{n_{xy,tor}}$	6.4 %	k_{mod}	0.90	at x y = 1.79 m 0.00 m	Fundamental combination: $q_d + H_d + q_{\perp,d}$
Buckling	$\eta_{buckling}$	57.4 %	k_{mod}	0.90	at x y = 3.58 m 1.43 m	Fundamental combination: $q_d + H_d + q_{\perp,d}$

Utilisation ratios in case of fire						
Bending with normal force $m_y + n_y$	$\eta_{m_y+n_y,fi}$	10 %	$k_{mod,fi}$	1.00	at x y = 3.58 m 1.43 m	Accidental combination: $q_{d,fi} + H_{d,fi} + q_{\perp,d,fi}$
Shear v_y	$\eta_{v_y,fi}$	0 %	$k_{mod,fi}$	1.00	at x y = 1.79 m 0.00 m	Accidental combination: $q_{d,fi} + H_{d,fi} + q_{\perp,d,fi}$
Shear force n_{xy} (gross shear)	$\eta_{n_{xy,gross,fi}}$	0 %	$k_{mod,fi}$	1.00	at x y = 1.79 m 0.00 m	Accidental combination: $q_{d,fi} + H_{d,fi} + q_{\perp,d,fi}$
Shear force n_{xy} (net shear)	$\eta_{n_{xy,net,fi}}$	0 %	$k_{mod,fi}$	1.00	at x y = 1.79 m 0.00 m	Accidental combination: $q_{d,fi} + H_{d,fi} + q_{\perp,d,fi}$
Shear force n_{xy} (torsion)	$\eta_{n_{xy,tor,fi}}$	0 %	$k_{mod,fi}$	1.00	at x y = 1.79 m 0.00 m	Accidental combination: $q_{d,fi} + H_{d,fi} + q_{\perp,d,fi}$
Buckling	$\eta_{buckling,fi}$	61.3 %	$k_{mod,fi}$	1.00	at x y = 3.58 m 1.43 m	Accidental combination: $q_{d,fi} + H_{d,fi} + q_{\perp,d,fi}$

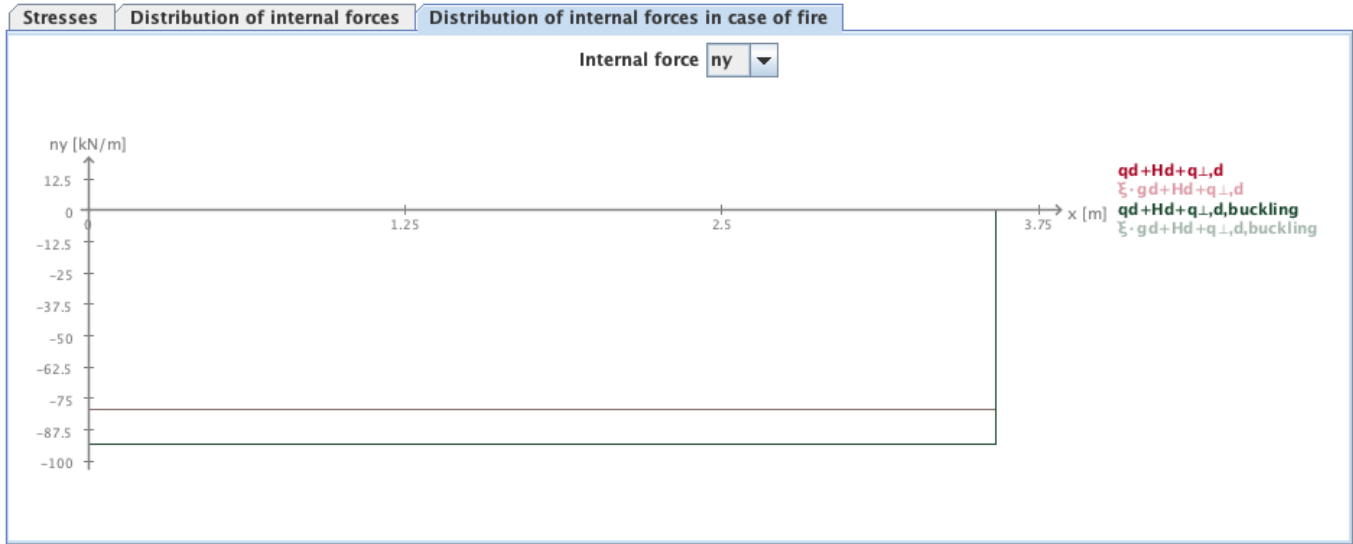
The normal and shear stresses are given in the tab "Stresses"



The distribution of the internal forces n_y and n_{xy} over the wall length and m_y and v_y over the wall height are given in the tab "Distribution of internal forces".



Analogously, the internal forces in case of fire.



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